Publication No. FHWA NHI 12-049 December, 2012



U.S. Department of Transportation

Federal Highway Administration

Bridge Inspector's Reference Manual



BIRM Volume 1





Technical Report Documentation Page

1.	Report No. FHWA NHJ 12-049	2. Government Accession No.	3.	Recipient's Catalog No.		
4.	Title and Subtitle		5.	Report Date		
	Bridge Inspector's Reference Manu	al (BIRM)		October 2002/November 2006/February 2012/July 2012/December 2012		
		6.	Performing Organization Code			
7.	Author (s) Thomas W. Ryan, P.E, J.	Eric Mann, P.E., Zachary M. Chill, E.I.T.,	8.	Performing Organization Report No.		
	Bryan T. Ott			23104 / 106915-HRS/118802		
9.	Performing Organization Name and Addu	ress	10.	Work Unit No. (TRAIS)		
	Michael Baker, Jr., Inc.					
	Airside Business Park, 100 Airside	Drive	11.	Contract or Grant No.		
	Moon Township, PA 15108			DTFH61-06-T-00009-T09-B07		
12.	Sponsoring Agency Name and Address		13.	Type of Report and Period Covered		
	Federal Highway Administration			Final Manual: October 2002		
	National Highway Institute (HNHI-	10)	1	Revised Manual: December 2006		
	1300 N. Courthouse Road			Revised Manual: February 2012		
	Arlington, Virginia 22201			Revised Manual: July 2012 Revised Manual: December 2012		
15	Supplementary Notes		14	Sponsoring Agoney Code		
13.	Baker Principle Investigator: Raym	and A. Hartle, P.F. (2002, 2006)	14.	Sponsoring Agency Code		
	Baker Principle Investigator: Mayin	P. Rosick, P.E. (2012)				
	Baker Project Manager: Thomas W	. Ryan, P.E. (2002, 2006, 2012)				
	FHWA Contracting Officer's Techn	nical Representative: Larry E. Jones (2002,				
	2006)					
	FHWA Contracting Officer's Tech	nical Representative : Louisa M. Ward, E.I.T.				
	Team Leader. Technical Review Te	am: John M. Hooks, P.E. (2002)				
	Team Leader, Technical Review Te	am: Thomas D. Everett, P.E (2006)				
	Team Leader, Technical Review Te	am: Gary E. Moss, P.E. (2012)				
16.	Abstract					
	This document, the <i>Bridge Inspector's Reference Manual (BIRM)</i> , is a comprehensive manual on programs, procedures, and techniques for inspecting and evaluating a variety of in-service highway bridges. It is intended to replace the <i>BITM 9</i> which was first published in 1991 to assist in training highway personnel for the new discipline of bridge safety inspectic <i>BITM 90</i> replaced <i>BITM 70</i> which had been in use for 20 years and has been the basis for several training programs varying in length from a few days to two weeks. Comprehensive supplements to <i>BITM 70</i> have been developed to cover inspection of fracture critical bridge members, and culverts are now covered in the <i>BIRM</i> .					
	The <i>BIRM</i> is a revision and upgrading of the previous manual. Improved Bridge Inspection techniques are presented, and state-of-the-art inspection equipment is included. New or expanded coverage is provided on culverts, fracture critical members, cable-stayed bridges, prestressed segmental bridges, moveable bridge inspection, underwater inspection, and non-destructive evaluation and critical findings.					
	A three-week comprehensive training program on bridge inspection, based on the <i>BIRM</i> , has been developed. The prograconsists of a one-week course, FHWA-NHI Course No 130054, "Engineering Concepts for Bridge Inspectors," and a two week course, FHWA-NHI Course No 130055, "Safety Inspection of In-Service Bridges." Together, these two courses m the definition of a comprehensive training program in bridge inspection as defined in the National Bridge Inspection Standards. Successful completion of the prerequisite FHWA-NHI Course No. 130054 is required to register for this FHWA-NHI 130055 Course. FHWA-NHI Course No. 130101, Introduction to Safety Inspection of In-Service Bridges, web based training or FHWA-NHI Course No. 130101a web based training assessment may be substituted for FHWA-NC Course No. 130054.					
	The catalogue for NHI Courses incl Highway Institute web site: <u>www.n</u>	uding the schedule, description and course req hi.fhwa.dot.gov	uest f	form can be found on the National		

17.	Key Words			18. Distribution Statement				
	Bridge Inspection, Bridge Evaluation, Element Level Evaluation, Component Rating, Culvert Inspection,			This report is available to the public from the				n the
	Critical Findings, Fracture Critical Members, Underwater			National Techni	cal Ir	nformation Ser	rvice	in
	Inspection			Springfield, Virginia 22161 and from the				
	1			Superintendent of Documents, U.S. Government			ernment	
				Printing Office,	Was	hington, D.C.	20402	2.
				-		-		
19.	Security Classif. (of this report)	20. Security Classif. (of th	is pag	e)	21.	No. of Pages	22.	Price
	Unclassified	Unclassified				1020		
-		D 1	0	1 . 1	•	1		

Form DOT F 1700.7 (8-72)

Reproduction of completed page authorized

ACKNOWLEDGEMENTS

We would like to express appreciation to the following individuals and organizations who contributed to the development and review of the BITM 90 Manual program:

Applied Science Associates, Inc.
Association of Diving Contractors, Inc. (Messrs. Castle, Harter, Hazelbaker, Hux, Maggard, McGeehan, and McGovern)
R. Richard Avent, Ph.D., P.E. (Louisiana State University)
William D. Domico, P.E. (Figg Engineering Group)
John W. Fisher, Ph.D. (Lehigh University)
Robert J. Hoyle, Jr., P.E., S.E.
Heinz P. Koretzky, P.E. (Pennsylvania DOT-Retired)
KTA-Tator, Inc.
John A. Schultz, Jr., S.E. (Hazelet and Erdal, Inc.)
Frieder Seible, Ph.D., P.E. (SEQAD Consulting Engineers, Inc.)
Daryl B. Simons, Ph.D., P.E. (Simons and Associates, Inc.)
Robert K. Simons, Ph.D., P.E. (Simons and Associates, Inc.)

Special thanks to the FHWA Technical Review Committee and NHI Instructional Development Review Committee:

John Bargo (2006) Steve Belcher (2002) Douglas Blades (2012) Gary Bowling (2012) Barry Brecto (2002, 2012) Eric Brown (2012) Shay Burrows (2006) Eric Christie (2012) Milo Cress (2002) Shelia Dawadi (2002) Timothy Earp (2012) Doug Edwards (2006) Thomas Everett (2006, 2012) Ken Foster (2012) Carl Highsmith (2006) John Hooks (2002) Calvin Karoer (2006) Greg Kolle (2006) Joe Krolak (2006) Gerald LaCosta (2012) Everett Matias (2006) Gary Moss (2006, 2012) Raymond McCormick (2002)

Curtis Monk (2002) Claude Napier (2006) Larry O'Donnell (2006, 2012) Jorge Pagan (2002) Meredith Perkins (2012) George Romack (2002) Luc Saroufim (2006) Lou Triandafilou (2002) John Thiel (2002) Louisa M. Ward (2012) Glenn Washer (2002) Robert Zaffetti (2012)

We would also like to thank the following individuals and organizations for providing information or visual aids for the BITM 90 Manual:

Kathleen H. Almand (American Iron and Steel Institute) American Traffic Safety Services Association Bridge Grid Flooring Manufacturers Association The D.S. Brown Company Concrete Reinforcing Steel Institute Cosmec, Inc. Barry Dickson (West Virginia University) Dynamic Isolation Systems, Inc. Carl Edwards (Maine DOT) **Elgard** Corporation Thomas D. Everett (FHWA) Exodermic Bridge Deck Institute FHWA NDE Center Paul S. Fisk (NDT Corporation) Geerhard Haaijer, Ph.D. (American Institute of Steel Construction, Inc.) John M. Hanson, Ph.D. (Wiss, Janney, Elster Associates, Inc.) Richard P. Knight, P.E. (Dynamic Isolation Systems, Inc.) Louisiana Department of Transportation and Development Merriman, Inc. William R. Mochel, P.E. (Illinois Department of Transportation) Pennsylvania Department of Transportation Professional Service Industries, Inc.-Pittsburgh Testing Laboratory Division Schupack Suarez Engineers, Inc. David Severns, P.E. (Nevada DOT) Turner-Fairbanks (FHWA) Paul W. Verrill (Maine Department of Transportation) Watson Bowman Associates, Inc. Stewart C. Watson (Stafford/Watson, Inc.) Western Wood Structures, Inc. Weston Geophysical Corporation

We would like to acknowledge the contributing staff for the BITM 90 and FHWA-NHI 130055:

Edward J. Adamczyk, P.E. William J. Amrhein, P.E. Dennis R. Baughman, P.E. Robert W. Bondi, P.E. Jeffery J. Campbell, P.E. Paul L. Coblentz, P.E. Raymond A. Hartle, P.E. Thomas L. Hooks Stephen J. Hoyer Gerald J. Jones, P.E. Guy R. Lang, Karen G. Lucci Thomas P. Macioce Susan Maines-Harrison J. Eric Mann, P.E. Robert M. Marchak Kelly A. McKindley Krisha E. Medziuch Edward E. Moldovan Charles L. Molnar Charles M. Schubert, P.E. J. Keith Seibel John J. Tkacs, P.E. Terrence J. Tiberio, P.E. Scott D. Vannoy, P.E. Terri A. Vojnovich Ruth Williams Kenneth E. Wilson, III, P.E., S.E. Lori A. Wittebort

We would like to acknowledge the contributing staff for the 2002 BIRM and FHWA-NHI 130055:

Justin W. Bouscher, P.E. Jeffery J. Campbell, P.E. David Chang Karen D. Ciccone Maureen Kanfoush Linda M. Ketterer Patrick A Leach, P.E. Guy R. Lang, P.E. J. Eric Mann, P.E. Linda Montagna Sean A. Patrick, P.E. Timothy J. Pintar, P.E. Heather Proya James Shroads William B. Sosko Jorge M. Suarez, P.E. Scott D. Vannoy, P.E. Laura E. Volle Kenneth E. Wilson, III, P.E., S.E.

We would like to acknowledge the contributing staff for the 2006 BIRM and FHWA-NHI 130055:

Justin W. Bouscher, P.E. Jeffery J. Campbell, P.E. Karen D. Ciccone Maureen Kanfoush J. Eric Mann, P.E. Linda Montagna Joseph R. McKool, P.E. Mary P. Rosick, P.E. Scott D. Vannoy, P.E Kenneth E. Wilson III, P.E., S.E. We would like to acknowledge the contributing staff for the 2012 BIRM and FHWA-NHI 130055:

Sherry Bellan Jeffrey J. Campbell, P.E. Karen D. Ciccone Phillip Fish, P.E. Sandy F. Fitzgerald Ronald Gardner, P.E. Tom Harvey Edward Hilker Maureen Kanfoush Matthew A. Kolis, P.E. Guy R. Lang, P.E. Carol A. Palmer Heather Partozoti Eric Schiemer Richard M. Schoedel, P.E. Weidong Shi Scott D. Vannoy, P.E. Kenneth E. Wilson III, P.E., S.E.

Publication No. FHWA NHI 12-050 December, 2012



U.S. Department of Transportation

Federal Highway Administration

Bridge Inspector's Reference Manual



BIRM Volume 2





Technical Report Documentation Page

1.	Report No. FHWA NHI 12-050	2. Government Accession No.	3.	Recipient's Catalog No.	
4.	Title and Subtitle	1	5.	Report Date	
	Bridge Inspector's Reference Manu	al (BIRM)		October 2002/November 2006/February 2012/July 2012/December 2012	
		6.	Performing Organization Code		
7.	Author (s) Thomas W. Ryan, P.E, J.	Eric Mann, P.E., Zachary M. Chill, E.I.T.,	8.	Performing Organization Report No.	
	Bryan T. Ott			23104 / 106915-HRS/118802	
9.	Performing Organization Name and Add	ress	10.	Work Unit No. (TRAIS)	
	Michael Baker, Jr., Inc.				
	Airside Business Park, 100 Airside	Drive	11.	Contract or Grant No.	
	Moon Township, PA 15108			DTFH61-06-T-00009-T09-B07	
12.	Sponsoring Agency Name and Address		13.	Type of Report and Period Covered	
	Federal Highway Administration			Final Manual: October 2002	
	National Highway Institute (HNHI-	10)		Revised Manual: December 2006	
	1300 N. Courthouse Road			Revised Manual: February 2012	
	Arlington, Virginia 22201			Revised Manual: July 2012 Revised Manual: December 2012	
15	Supplementary Notes		14	Sponsoring Agongy Code	
13.	Baker Principle Investigator: Raym	and A. Hartle, P.F. (2002, 2006)	14.	Sponsoring Agency Couc	
	Baker Principle Investigator: May	P. Rosick, P.E. (2012)			
	Baker Project Manager: Thomas W	. Ryan, P.E. (2002, 2006, 2012)			
	FHWA Contracting Officer's Techn	nical Representative: Larry E. Jones (2002,			
	2006)				
	FHWA Contracting Officer's Tech	nical Representative : Louisa M. Ward, E.I.T.			
	Team Leader Technical Review Te	am: John M. Hooks, P.E. (2002)			
	Team Leader, Technical Review Te	cam: Thomas D. Everett, P.E (2002)			
	Team Leader, Technical Review Te	am: Gary E. Moss, P.E. (2012)			
16.	Abstract				
	This document, the <i>Bridge Inspecto</i> and techniques for inspecting and ev which was first published in 1991 to <i>BITM 90</i> replaced <i>BITM 70</i> which H varying in length from a few days to inspection of fracture critical bridge	<i>r's Reference Manual (BIRM)</i> , is a comprehenvaluating a variety of in-service highway bridg to assist in training highway personnel for the n nad been in use for 20 years and has been the botwo weeks. Comprehensive supplements to <i>E</i> members, and culverts are now covered in the	sive ges. It ew d asis f <i>BITM</i> e <i>BIR</i>	manual on programs, procedures, is intended to replace the <i>BITM 90</i> iscipline of bridge safety inspection. For several training programs 70 have been developed to cover <i>M</i> .	
	The <i>BIRM</i> is a revision and upgrading of the previous manual. Improved Bridge Inspection techniques are presented, and state-of-the-art inspection equipment is included. New or expanded coverage is provided on culverts, fracture critical members, cable-stayed bridges, prestressed segmental bridges, moveable bridge inspection, underwater inspection, and non-destructive evaluation and critical findings.				
	A three-week comprehensive training program on bridge inspection, based on the <i>BIRM</i> , has been developed. The progra consists of a one-week course, FHWA-NHI Course No 130054, "Engineering Concepts for Bridge Inspectors," and a tw week course, FHWA-NHI Course No 130055, "Safety Inspection of In-Service Bridges." Together, these two courses m the definition of a comprehensive training program in bridge inspection as defined in the National Bridge Inspection Standards. Successful completion of the prerequisite FHWA-NHI Course No. 130054 is required to register for this FHWA-NHI 130055 Course. FHWA-NHI Course No. 130101, Introduction to Safety Inspection of In-Service Bridges, web based training or FHWA-NHI Course No. 130101a web based training assessment may be substituted for FHWA-NC Course No. 130054.				
	The catalogue for NHI Courses including the schedule, description and course request form can be found Highway Institute web site: <u>www.nhi.fhwa.dot.gov</u>				

17.	Key Words			18. Distribution Statement				
	Bridge Inspection, Bridge Evaluation, Element Level Evaluation, Component Rating, Culvert Inspection,			This report is available to the public from the				n the
	Critical Findings, Fracture Critical	Members, Underwater		National Techni	cal Ir	nformation Ser	rvice i	n
	Inspection			Springfield, Virginia 22161 and from the				
	1			Superintendent of Documents, U.S. Government			ernment	
				Printing Office,	Was	hington, D.C.	20402	2.
19.	Security Classif. (of this report)	20. Security Classif. (of th	is pag	e)	21.	No. of Pages	22.	Price
	Unclassified	Unclassifie				984		
-			0	1 . 1 . 1	•			

Form DOT F 1700.7 (8-72)

Reproduction of completed page authorized

ACKNOWLEDGEMENTS

We would like to express appreciation to the following individuals and organizations who contributed to the development and review of the BITM 90 Manual program:

Applied Science Associates, Inc.
Association of Diving Contractors, Inc. (Messrs. Castle, Harter, Hazelbaker, Hux, Maggard, McGeehan, and McGovern)
R. Richard Avent, Ph.D., P.E. (Louisiana State University)
William D. Domico, P.E. (Figg Engineering Group)
John W. Fisher, Ph.D. (Lehigh University)
Robert J. Hoyle, Jr., P.E., S.E.
Heinz P. Koretzky, P.E. (Pennsylvania DOT-Retired)
KTA-Tator, Inc.
John A. Schultz, Jr., S.E. (Hazelet and Erdal, Inc.)
Frieder Seible, Ph.D., P.E. (SEQAD Consulting Engineers, Inc.)
Daryl B. Simons, Ph.D., P.E. (Simons and Associates, Inc.)
Robert K. Simons, Ph.D., P.E. (Simons and Associates, Inc.)

Special thanks to the FHWA Technical Review Committee and NHI Instructional Development Review Committee:

John Bargo (2006) Steve Belcher (2002) Douglas Blades (2012) Gary Bowling (2012) Barry Brecto (2002, 2012) Eric Brown (2012) Shay Burrows (2006) Eric Christie (2012) Milo Cress (2002) Shelia Dawadi (2002) Timothy Earp (2012) Doug Edwards (2006) Thomas Everett (2006, 2012) Ken Foster (2012) Carl Highsmith (2006) John Hooks (2002) Calvin Karoer (2006) Greg Kolle (2006) Joe Krolak (2006) Gerald LaCosta (2012) Everett Matias (2006) Gary Moss (2006, 2012) Raymond McCormick (2002)

Curtis Monk (2002) Claude Napier (2006) Larry O'Donnell (2006, 2012) Jorge Pagan (2002) Meredith Perkins (2012) George Romack (2002) Luc Saroufim (2006) Lou Triandafilou (2002) John Thiel (2002) Louisa M. Ward (2012) Glenn Washer (2002) Robert Zaffetti (2012)

We would also like to thank the following individuals and organizations for providing information or visual aids for the BITM 90 Manual:

Kathleen H. Almand (American Iron and Steel Institute) American Traffic Safety Services Association Bridge Grid Flooring Manufacturers Association The D.S. Brown Company Concrete Reinforcing Steel Institute Cosmec, Inc. Barry Dickson (West Virginia University) Dynamic Isolation Systems, Inc. Carl Edwards (Maine DOT) **Elgard** Corporation Thomas D. Everett (FHWA) Exodermic Bridge Deck Institute FHWA NDE Center Paul S. Fisk (NDT Corporation) Geerhard Haaijer, Ph.D. (American Institute of Steel Construction, Inc.) John M. Hanson, Ph.D. (Wiss, Janney, Elster Associates, Inc.) Richard P. Knight, P.E. (Dynamic Isolation Systems, Inc.) Louisiana Department of Transportation and Development Merriman, Inc. William R. Mochel, P.E. (Illinois Department of Transportation) Pennsylvania Department of Transportation Professional Service Industries, Inc.-Pittsburgh Testing Laboratory Division Schupack Suarez Engineers, Inc. David Severns, P.E. (Nevada DOT) Turner-Fairbanks (FHWA) Paul W. Verrill (Maine Department of Transportation) Watson Bowman Associates, Inc. Stewart C. Watson (Stafford/Watson, Inc.) Western Wood Structures, Inc. Weston Geophysical Corporation

We would like to acknowledge the contributing staff for the BITM 90 and FHWA-NHI 130055:

Edward J. Adamczyk, P.E. William J. Amrhein, P.E. Dennis R. Baughman, P.E. Robert W. Bondi, P.E. Jeffery J. Campbell, P.E. Paul L. Coblentz, P.E. Raymond A. Hartle, P.E. Thomas L. Hooks Stephen J. Hoyer Gerald J. Jones, P.E. Guy R. Lang, Karen G. Lucci Thomas P. Macioce Susan Maines-Harrison J. Eric Mann, P.E. Robert M. Marchak Kelly A. McKindley Krisha E. Medziuch Edward E. Moldovan Charles L. Molnar Charles M. Schubert, P.E. J. Keith Seibel John J. Tkacs, P.E. Terrence J. Tiberio, P.E. Scott D. Vannoy, P.E. Terri A. Vojnovich Ruth Williams Kenneth E. Wilson, III, P.E., S.E. Lori A. Wittebort

We would like to acknowledge the contributing staff for the 2002 BIRM and FHWA-NHI 130055:

Justin W. Bouscher, P.E. Jeffery J. Campbell, P.E. David Chang Karen D. Ciccone Maureen Kanfoush Linda M. Ketterer Patrick A Leach, P.E. Guy R. Lang, P.E. J. Eric Mann, P.E. Linda Montagna Sean A. Patrick, P.E. Timothy J. Pintar, P.E. Heather Proya James Shroads William B. Sosko Jorge M. Suarez, P.E. Scott D. Vannoy, P.E. Laura E. Volle Kenneth E. Wilson, III, P.E., S.E.

We would like to acknowledge the contributing staff for the 2006 BIRM and FHWA-NHI 130055:

Justin W. Bouscher, P.E. Jeffery J. Campbell, P.E. Karen D. Ciccone Maureen Kanfoush J. Eric Mann, P.E. Linda Montagna Joseph R. McKool, P.E. Mary P. Rosick, P.E. Scott D. Vannoy, P.E Kenneth E. Wilson III, P.E., S.E. We would like to acknowledge the contributing staff for the 2012 BIRM and FHWA-NHI 130055:

Sherry Bellan Jeffrey J. Campbell, P.E. Karen D. Ciccone Phillip Fish, P.E. Sandy F. Fitzgerald Ronald Gardner, P.E. Tom Harvey Edward Hilker Maureen Kanfoush Matthew A. Kolis, P.E. Guy R. Lang, P.E. Carol A. Palmer Heather Partozoti Eric Schiemer Richard M. Schoedel, P.E. Weidong Shi Scott D. Vannoy, P.E. Kenneth E. Wilson III, P.E., S.E.

BI	RM
Bridge Inspector's Reference Manual	Bridge Inspector's Reference Manual
December, 2012 FHWA NHI Publication No. 12-049 Volume 1	December, 2012 FHWA NHI Publication No. 12-050 Volume 2

Table of Contents

TOPIC

Chapter 1: Bridge Inspection Programs

- 1.1 History of the National Bridge Inspection Program
- 1.2 Responsibilities of the Bridge Inspector
- 1.3 Quality Control and Quality Assurance

Chapter 2: Safety Fundamentals for Bridge Inspectors

- 2.1 Duties of the Bridge Inspection Team
- 2.2 Safety Fundamentals for Bridge Inspectors
- 2.3 Temporary Traffic Control
- 2.4 Inspection Equipment
- 2.5 Methods of Access

Chapter 3: Basic Bridge Terminology

3.1 Basic Bridge Terminology

Chapter 4: Bridge Inspection Reporting

- 4.1 Structure Inventory
- 4.2 Condition and Appraisal
- 4.3 Introduction to Element Level Evaluation
- 4.4 Record Keeping and Documentation
- 4.5 Critical Findings
- 4.6 The Inspection Report

Chapter 5: Bridge Mechanics

5.1 Bridge Mechanics

TOPIC

Chapter 6: Bridge Materials

- 6.1 Timber
- 6.2 Concrete
- 6.3 Steel/ Metal
- 6.4 Fatigue and Facture in Steel
- 6.5 Stone Masonry
- 6.6 Fiber Reinforced Polymer(FRP)

Chapter 7: Inspection and Evaluation Bridges Decks and Areas Adjacent To Bridge Decks

- 7.1 Timber Decks
- 7.2 Concrete Decks
- 7.3 Fiber Reinforced Polymer (FRP) Decks
- 7.4 Steel Decks
- 7.5 Deck Joints, Drainage Systems, Lighting and Signs
- 7.6 Safety Features

Chapter 8: Inspection and Evaluation of Timber Superstructures

- 8.1 Solid Sawn Timber Bridges
- 8.2 Glulam Timber Bridges
- 8.3 Stress-Laminated Timber Bridges

Chapter 9: Inspection and Evaluation of Concrete Superstructures

- 9.1 Cast-In-Place Slabs
- 9.2 Cast-In-Place Tee Beams
- 9.3 Conventionally Reinforced Concrete Two-Girder System
- 9.4 Concrete Channel Beams
- 9.5 Concrete Arches
- 9.6 Concrete Rigid Frames
- 9.7 Precast and Prestressed Slabs
- 9.8 Prestressed Double Tees
- 9.9 Prestressed I-Beams and Bulb Tees
- 9.10 Prestressed Box Beams
- 9.11 Concrete Box Girders

TOPIC

Chapter 10: Inspection and Evaluation of Steel Superstructures

- 10.1 Rolled Steel Multi-Beams and Fabricated Steel Multi-Girders
- 10.2 Steel Two-Girder Systems
- 10.3 Steel Box Beams and Girders
- 10.4 Steel Trusses
- 10.5 Steel Arches
- 10.6 Steel Rigid Frames
- 10.7 Pin-and-Hanger Assemblies
- 10.8 Gusset Plates
- 10.9 Steel Eyebars

Chapter 11: Inspection and Evaluation of Bridge Bearings

11.1 Bridge Bearings

Chapter 12: Inspection and Evaluation of Substructures

- 12.1 Abutments and Wingwalls
- 12.2 Piers and Bents

Chapter 13: Inspection and Evaluation of Waterways

- 13.1 Waterway Elements
- 13.2 Inspection of Waterways
- 13.3 Underwater Inspection

Chapter 14: Characteristics, Inspection and Evaluation of Culverts

- 14.1 Culvert Characteristics
- 14.2 Rigid Culverts
- 14.3 Flexible Culverts

Chapter 15: Advanced Inspection Methods

- 15.1 Timber
- 15.2 Concrete
- 15.3 Steel
- 15.4 Advanced Bridge Evaluation

Chapter 16: Complex Bridges

- 16.1 Cable-Supported Bridges
- 16.2 Movable Bridges
- 16.3 Floating Bridges

Comprehensive Bridge Safety Inspection Training Program Bridge Inspector's Reference Manual BIRM 2002 Revised 2006 Revised 2012

Appendix A: Sample Inspection Report

Appendix B: National Bridge Inspection Standards

Glossary

List of Figures

	Figure Nos.		Page Nos.
Topic 1.1 History of the National Bridge Inspection Program			
	1.1.1	Number of Bridges Built since 1900	1.1.2
	1.1.2	Collapse of the Silver Bridge	1.1.2
	1.1.3	Federal Funding Levels $(19/9 - 2003)$	1.1.0
Topic 1.2 Responsibilities of the Bridge Inspector			
•	1.2.1	Mianus Bridge Failure	1.2.2
Topic 1.3 Quality Control and Quality Assurance			
		None	
Topic 2.1 Duties of the Bridge Inspection Team			
	2.1.1	Sample Bridge Numbering Sequence	2.1.3
	2.1.2	Sample Truss Numbering Scheme	2.1.4
	2.1.3	Sample Inspection Sequence	2.1.6
	2.1.4 2.1.5	Inspection for Scour and Undermining	2.1./
	<u> </u>	inspection for beout and ondernining	2.1.12

Figure			
Nos.			

Page Nos.

Topic 2.2 Safety Fundamentals for Bridge Inspectors

Inspector Wearing a Hard Hat	2.2.3
Inspector Wearing A Reflective Safety Vest	2.2.4
Inspector Wearing Safety Goggles and Gloves	2.2.5
Inspector Wearing a Life Jacket	2.2.5
Inspector Wearing a Respirator	2.2.6
Inspector with Safety Harness with a Lanyard	2.2.7
Inspection Involving Extensive Climbing	2.2.11
Proper Use of Ladder	2.2.12
Bucket Truck	2.2.13
Inspection Catwalk	2.2.14
Inspection Rigging	2.2.15
Inclement Weather Causing Slippery Bridge	
Members and Poor Visibility for Motorists	2.2.18
	Inspector Wearing a Hard Hat Inspector Wearing A Reflective Safety Vest Inspector Wearing Safety Goggles and Gloves Inspector Wearing a Life Jacket Inspector Wearing a Respirator Inspector Wearing a Respirator Inspector with Safety Harness with a Lanyard Inspector Involving Extensive Climbing Proper Use of Ladder Bucket Truck Inspection Catwalk Inspection Rigging Inclement Weather Causing Slippery Bridge Members and Poor Visibility for Motorists

Topic 2.3 Temporary Traffic Control

2.3.1	Temporary Traffic Control Operation	2.3.1
2.3.2	Work Zone	2.3.3
2.3.3	Inspection Vehicles with Flashing Light	2.3.4
2.3.4	Regulatory Sign	2.3.7
2.3.5	Warning Sign	2.3.7
2.3.6	Examples of Guide Signs	2.3.7
2.3.7	Arrow Board	2.3.8
2.3.8	Changeable Message Sign	2.3.8
2.3.9	Cones	2.3.10
2.3.10	Drums	2.3.10
2.3.11	Tubular Marker	2.3.11
2.3.12	Vertical Panel	2.3.11
2.3.13	Temporary Traffic Barriers	2.3.12
2.3.14	Warning Lights	2.3.13
2.3.15	Use of Hand Signaling Devices by Flagger (from	
	Manual on Uniform Traffic Control Devices	
	(MUTCD))	2.3.15
2.3.16	Stopping Sight Distance as a Function of Speed	
	(from Manual on Uniform Traffic Control Devices	
	(MUTCD))	2.3.16
2.3.17	Flagger with Stop/Slow Paddle	2.3.17

Figure		Page
Nos.		Nos.
2.3.18	Shadow Vehicle with Attenuator	2.3.19

Topic 2.4 Inspection Equipment

2.4.1	Tools for Cleaning	2.4.1
2.4.2	Tools for Inspection	2.4.2
2.4.3	Tools for Visual Aid	2.4.2
2.4.4	Tools for Measuring	2.4.3
2.4.5	Rotary Percussion	2.4.6
2.4.6	Scour Monitoring Collar	2.4.8
2.4.7	Scour Monitoring Collar Schematic	2.4.9
2.4.8	Remote Camera	2.4.10
2.4.9	High Speed Underclearance Measurement System	2.4.11
2.4.10	Tablet PC Used to collect inspection data	2.4.12

Topic 2.5 Methods of

Access

2.5.1	Inspectors using a Ladder with the Proper 1H to 4V	2.5.1
	Ratio	
2.5.2	Inspector using a Hook-ladder	2.5.2
2.5.3	Rigging for Substructure Inspection	2.5.3
2.5.4	Rigging for Superstructure Inspection	2.5.3
2.5.5	Scaffold	2.5.4
2.5.6	Inspection Operations from a Barge	2.5.5
2.5.7	Climber	2.5.5
2.5.8	Inspector Using Float	2.5.6
2.5.9	Inspector Rappelling Substructure Unit	2.5.6
2.5.10	Climbing	2.5.7
2.5.11	Catwalk	2.5.8
2.5.12	Traveler Platform	2.5.9
2.5.13	Handrail on Girder Bridge	2.5.10
2.5.14	Handrail on Suspension Bridge	2.5.10
2.5.15	Manlift	2.5.11
2.5.16	Scissor Lift	2.5.12
2.5.17	Bucket Truck	2.5.13
2.5.18	Track-mounted Man-lift in a Stream	2.5.13
2.5.19	Track-mounted Man-lift on a Slope	2.5.14
2.5.20	Under Bridge Inspection Vehicle with Bucket	2.5.14
2.5.21	Under Bridge Inspection Vehicle with Platform	2.5.15

Figure Nos.

Topic 3.1 Basic Bridge Terminology

211	NDIS Structure I on oth	211
3.1.1	NBIS Bridge Length (Coding Guide Item 112)	3.1.1 3.1.2
212	Major Pridge Components	2.1.2
3.1.3 2.1.4	Timber Members	3.1.3 2.1.4
2.1. 4	Timber Dooma	3.1. 4 2.1.5
5.1.5 2.1.6	Laugual Canarata Shanag	5.1.5 2 1 5
5.1.0 2.1.7	Dainformed Concrete Shapes	3.1.3 2.1.6
3.1./ 2.1.0	Prostrogged Concrete Shapes	3.1.0 2 1 7
5.1.0 2.1.0	New greatenessed Wild Steel Deinforged Concrete vie	3.1./
3.1.9	Non-prestressed Mild Steel Reinforced Concrete Vs.	210
2 1 10	Compareta Dila Dant	3.1.0
5.1.10 2 1 11	Steel Melting Organization	5.1.9 2 1 1 1
3.1.11 2.1.12	Common Dollad Steel Shares	3.1.11
3.1.12 2.1.12	Common Kolled Steel Snapes	3.1.12
3.1.13	Bracing Members Made from Angles, Bars, and Plates	3.1.13
3.1.14	Riveted Plate Girder	3.1.15
3.1.15	Riveted Box Shapes	3.1.15
3.1.16	Welded I-Beam	3.1.16
3.1.17	Welded Box Shapes	3.1.16
3.1.18	Cable Cross-Sections	3.1.17
3.1.19	Cable-Supported Bridge: Suspension Cables and	
	Hangers	3.1.17
3.1.20	Cable-Supported Bridge: Cable Stayed	3.1.18
3.1.21	Sizes of Bridge Pins	3.1.19
3.1.22	Pin-Connected Eyebars	3.1.19
3.1.23	Types of Rivet Heads	3.1.20
3.1.24	Shop Rivets and Field Bolts	3.1.21
3.1.25	Close-up of Tack Weld on a Riveted Built-up Truss	
	Member	3.1.22
3.1.26	Pin and Hanger Assembly	3.1.23
3.1.27	Bolted Field Splice	3.1.24
3.1.28	Bridge Deck with a Smooth Riding Surface	3.1.24
3.1.29	Underside View of a Bridge Deck	3.1.25
3.1.30	Composite Deck and Steel Superstructure	3.1.26
3.1.31	Shear Studs on Top Flange of Girder (before	
	Concrete Deck is Placed)	3.1.26
3.1.32	Plank Deck	3.1.27
3.1.33	Concrete Deck	3.1.28
3.1.34	Steel Grid Deck	3.1.28
		-
3.1.35	Fiber Reinforced Polymer (FRP) Deck	3.1.29

Page Nos.

Figure		Page
Nos.		Nos.
3.1.36	Asphalt Wearing Surface on a Concrete Deck	3.1.30
3.1.37	Strip Seal Expansion Joint	3.1.31
3.1.38	Top View of an Armored Compression Seal in	3.1.32
	Place	
3.1.39	Top View of a Finger Plate Joint	3.1.32
3.1.40	New Jersey Barrier	3.1.34
3.1.41	Weight Limit Sign and Object Marker Signs	3.1.35
3.1.42	Bridge Lighting	3.1.35
3.1.43	Four Basic Bridge Types	3.1.36
3.1.44	Slab Bridge	3.1.37
3.1.45	Beam Bridge	3.1.37
3.1.46	Multi-Girder Bridge	3.1.38
3.1.47	Girder Floorbeam Stringer Bridge	3.1.38
3.1.48	Curved Girder Bridge	3.1.39
3.1.49	Tee Beam Bridge	3.1.39
3.1.50	Adjacent Box Beam Bridge	3.1.40
3.1.51	Box Girder Bridge	3.1.40
3.1.52	Deck Truss Bridge	3.1.41
3.1.53	Through Truss Bridge	3.1.41
3.1.54	Deck Arch Bridge	3.1.42
3.1.55	Through Arch Bridge	3.1.42
3.1.56	Rigid Frame	3.1.43
3.1.57	Suspension Bridge	3.1.44
3.1.58	Cable-stayed Bridge	3.1.44
3.1.59	Bascule Bridge	3.1.45
3.1.60	Swing Bridge	3.1.45
3.1.61	Lift Bridge	3.1.46
3.1.62	Floating Bridge	3.1.46
3.1.63	Floor System and Main Supporting Members	3.1.47
3.1.64	Main Supporting Members of Deck Arch	3.1.48
3.1.65	Diaphragms	3.1.48
3.1.66	Cross or X-Bracing	3.1.49
3.1.67	Top Lateral Bracing and Sway Bracing	3.1.49
3.1.68	Steel Roller Bearing Showing Four Basic Elements	3.1.51
3.1.69	Abutment	3.1.52
3.1.70	Pier	3.1.52
3.1.71	Cantilever Abutment (or Full Height Abutment)	3.1.53
3.1.72	Stub Abutment	3.1.54
3.1.73	Spill-Through or Open Abutment	3.1.54
3.1.74	Integral Abutment	3.1.55
3.1.75	Solid Shaft Pier	3.1.56
3.1.76	Solid Shaft Pier	3.1.56
3.1.77	Column Pier	3.1.57

Figure Nos.		Page Nos.
3.1.78	Column Pier with Web Wall and Cantilevered Pier	3.1.57
	Caps	
3.1.79	Cantilever or Hammerhead Pier	3.1.58
3.1.80	Column Bent	3.1.58
3.1.81	Pile Bent	3.1.59
3.1.82	Schematic of Common Abutment Types	3.1.60
3.1.83	Rigid Culvert	3.1.62
3.1.84	Flexible Culvert	3.1.62

Topic 4.1 Structure Inventory

4.1.1	Example SI&A Sheet with Element Level Data	4.1.3
4.1.2	Typical SI&A Sheet with NBI Data Only	4.1.4
4.1.3	Oregon Bridge Inspection Report with Element	4.1.5
	Level Data	
4.1.4	Arizona Structural Inventory and Appraisal Sheet	4.1.7
4.1.5	Florida Structural Inventory and Appraisal Sheet	4.1.8
4.1.6	Portable Computer	4.1.12
4.1.7	Inspector Using Portable Computer	4.1.12
4.1.8	Wearable Computer with Case	4.1.13
4.1.9	Inspector Using Wearable Computer	4.1.13

Topic 4.2 Condition and Appraisal

None

Topic 4.3 Introduction to Element Level Evaluation

4.3.1	AASHTO Guide Manual for Bridge Element	4.3.4
	Inspection	
4.3.2	Decks/Slabs National Bridge Elements in the	
	AASHTO Guide	4.3.6
4.3.3	Superstructure National Bridge Elements in the	
	AASHTO Guide	
		4.3.7
4.3.4	Substructure National Bridge Elements in the	
	AASHTO Guide	4.3.7
4.3.5	Decks/Slabs Bridge Management Elements in the	
	AASHTO	4.3.8

Figure		Page
Nos.		Nos.
4.3.6	Wearing Surfaces and Protective Systems in the	
	AASHTO Guide	4.3.8

Topic 4.4 Record Keeping			
anu Documentation			
Documentation	441	Sample Photo Log	443
	т.т.1 ДДЭ	Accident Involving Construction Equipment and a	т.т.у
	7.7.2	Bridge	ΔΔΔ
	443	Posted Bridge	444
	т.т. <i>3</i> ДДД	Flood Event	т.т.т 4 4 5
	т.т.т ДД 5	Flement I evel Example Inspection Form	л.н. <i>3</i> ДД 8
	л.н. <i>5</i> ДДД	Example Load Rating Summary Sheet	4.4.0 4.4.0
	ч.ч.0 ДД 7	Inspector Taking Notes	4 4 10
	т.т. / ДД 8	Flectronic Data Collection	4.4.11
	ч.ч.0 ДД 0	Sample Span Numbering Scheme	<i>A A</i> 13
	4 4 10	Sample Spain Rundering Scheme	4 4 13
	4.4.10 4.4.11	Sample Structure Orientation Sketch	4 4 13
	л.т.11 ДД12	Sample Truss Numbering Scheme	4 A 1 A
	4 A 13	Steel Superstructure Dimensions	<u> </u>
	л.н.1 <i>3</i> ДД 1Д	Truss Member and Field Splice Dimensions	<i>A A</i> 15
	л.т. 1 л Л Л 15	Framing Plan	л.н. Л Л 16
	4 4 16	Girder Flevation	4.4.17
	4.4.17	Typical Prenared Culvert Sketches	4 4 17
	ΔΔ18	Sample General Plan Sketch	<u> </u>
	4.4.10 4.4.10	Sample General Flevation Sketch	4 4 18
	4 4 20	Sample Deck Inspection Notes	4 4 20
	4.4.20 4.4.21	Sample Superstructure Inspection Notes	4 4 21
	4 4 22	Sample Substructure Inspection Notes	4 4 22
	т.т.22 ДД 23	Sample Channel Inspection Notes	4 4 7 2
	т.т. <i>23</i> 4 4 74	Example Inspection Form – PennDOT Form D-450	4 4 73
	1.7.47	Example inspection Form Femileo From D-450	г.т. <i>2</i> Ј

Topic 4.5 Critical Findings

4.5.1 4 5 2	Missouri DOT Critical Inspection Finding Form 4 Washington State DOT "Critical Damage - Bridge	1.5.3
1.0.2	Repair Report"	1.5.4
4.5.3	Pennsylvania DOT Critical and High Priority	
	Maintenance Items – Flowchart for Plan of Action 4	4.5.10
4.5.4	Washington State DOT Flowchart for Field	
	Inspection Procedure 4	4.5.11

Fi _i N	gure Pa Nos. N	age los.
Topic 4.6 Inspection Report		
4.6	6.1 Near Approach - Toward Bridge 4.6	.6
4.6	5.2 Downstream Elevation	.6
Topic 5.1 Bridge Mechanics		
5.1	.1 Permanent Load on a Bridge 5.1	.2
5.1	.2 Vehicle Transient Load on a Bridge	.3
5.1	.3 AASHTO H20 Truck	.4
5.1	.4 AASHTO HS20 Truck 5.1	.5
5.1	.5 AASHTO Lane Loadings	.6
5.1	.6 AASHTO LRFD Loading	.7
5.1	.7 Alternate Military Loading	.7
5.1	.8 Permit Vehicle	.8
5.1	.9 Axial Forces 5.1	.10
5.1	.10 Positive and Negative Moment	.12
5.1	.11 Girder Cross Section Resisting Positive Moment 5.1	.12
5.1	.12 Shear Forces in a Member Element 5.1	.13
5.1	.13 Torsion	.14
5.1	.14 Torsional Distortion 5.1	.15
5.1	.15 Types of Supports 5.1	.15
5.1	.16 Basic Force Components 5.1	.16
5.1	.17 Stress-Strain Diagram 5.1	.19
5.1	.18 Rating Vehicles	.25
5.1	.19 Bridge Weight Limit Posting	.26
5.1	.20 Damaged Bridge due to Failure to Comply with	-
-	Bridge Posting 5.1	.27
5.1	.21 Simple Bridge 5.1	.28
5.1	.22 Continuous Bridge 5.1	.29
5.1	.23 Cantilever Span 5.1	.30
5.1	.24 Cantilever Bridge 5.1	.31
5.1	.25 Non-Composite or Composite Concrete Deck on	
	Steel Beams and Pretressed Concrete Beams 5.1	.32
5.1	.26 Integral Bridge 5.1	.33
5.1	.27 Cross Section of an Integral Bridge	.33
5.1	.28 Orthotropic Bridge Deck	.34
5.1	20 Spread Facting 51	26
		.30

Figure	Page
Nos.	Nos.

Topic 6.1 Timber

6.1.1	Glued-laminated Modern Timber Bridge	6.1.2
6.1.2	Timber Shapes	6.1.3
6.1.3	Built-up Timber Shapes	6.1.3
6.1.4	Anatomy of Timber	6.1.5
6.1.5	3-D Close-up of Softwood Timber Anatomy	
	(Source: Society of Wood Science and Technology).	6.1.6
6.1.6	Three Principal Axes of Wood	6.1.7
6.1.7	Inherent Timber Defects	6.1.11
6.1.8	Decay of Wood by Fungi	6.1.11
6.1.9	Mold and Stain on Underside of Timber Bridge	6.1.13
6.1.10	Brown and White Rot	6.1.13
6.1.11	Termites	6.1.15
6.1.12	Powder Post Beetle	6.1.15
6.1.13	Carpenter Ants	6.1.16
6.1.14	Caddis fly Larva	6.1.17
6.1.15	Marine Borer Damage to Wood Piling	6.1.18
6.1.16	Shipworm (Mollusk)	6.1.18
6.1.17	Limnoria (Wood Louse)	6.1.19
6.1.18	Delamination in a Glue Laminated Timber Member	6.1.20
6.1.19	Loose Hanger Connection Between the Timber	
	Truss and Floorbeam	6.1.20
6.1.20	Fire Damaged Timber Members	6.1.21
6.1.21	Impact/Collision Damage to a Timber Member	6.1.22
6.1.22	Wear of a Timber Deck	6.1.22
6.1.23	Horizontal Shear Failure in Timber Member	6.1.23
6.1.24	Failed Timber Floor Beam due to Excessive	
	Bending Moment	6.1.23
6.1.25	Timber Substructure Member Subjected to	
	Crushing and Overstress	6.1.24
6.1.26	Weathering on Timber Deck	6.1.24
6.1.27	Bridge Timber Member Showing Penetration	
	Depth of Preservative Treatment	6.1.26
6.1.28	Coal-Tar Creosote Treated Timber Beams (Source:	
	Barry Dickson, West Virginia University)	6.1.26
6.1.29	Inspector Performing a Pick Test	6.1.30
6.1.30	Timber Boring and Drilling Locations	6.1.31
6.1.31	Gage Used to Measure Coating Dry Film Thickness.	6.1.32
-	0 0 1	

Topic 6.2 Concrete

6.2.1	Round Concrete Members	6.2.4
-------	------------------------	-------

Figure		Page
Nos.		Nos.
6.2.2	Rectangular Concrete Members	6.2.5
6.2.3	Strength Properties of Concrete (3500 psi Concrete).	6.2.6
6.2.4	FHWA's Strategic Highway Research Program	
	(SHRP) Implemented HPC Mix Designs	6.2.7
6.2.5	Concrete Member with Tensile Steel	
	Reinforcement Showing	6.2.8
6.2.6	Standard Deformed Reinforcing Bars (Source:	
	Concrete Reinforcing Steel Institute)	6.2.9
6.2.7	Standard Deformed Reinforcing Bars (Source:	
	Concrete Reinforcing Steel Institute) (Continued)	6.2.10
6.2.8	Prestressed Concrete Beam	6.2.12
6.2.9	Pretensioned Concrete I-Beams	6.2.13
6.2.10	Post-tensioned Concrete Box Girders	6.2.13
6.2.11	Structural Cracks	6.2.16
6.2.12	Flexural Crack on a Tee Beam	6.2.17
6.2.13	Shear Crack on a Slab	6.2.17
6.2.14	Crack Comparator Card	6.2.18
6.2.15	Temperature Cracks	6.2.20
6.2.16	Shrinkage Cracks	6.2.20
6.2.17	Transverse Cracks	6.2.22
6.2.18	Longitudinal Cracks	6.2.22
6.2.19	Pattern or Map Cracks	6.2.23
6.2.20	Light or Minor Scaling	6.2.24
6.2.21	Medium or Moderate Scaling	6.2.24
6.2.22	Heavy Scaling	6.2.24
6.2.23	Severe Scaling	6.2.24
6.2.24	Spalling on a Concrete Deck	6.2.25
6.2.25	Efflorescence	6.2.27
6.2.26	Alkali-Silica Reaction (ASR)	6.2.28
6.2.27	Honeycomb	6.2.29
6.2.28	Concrete Column Collision Damage	6.2.30
6.2.29	Substructure Abrasion	6.2.31
6.2.30	Overload Damage	6.2.32
6.2.31	Loss of Bond: Concrete / Corroded Reinforcing Bar	6.2.33
6.2.32	Anti-Graffiti Coating on Lower Area of Bridge	
	Piers	6.2.38
6.2.33	Inspector Using a Chain Drag	6.2.41

Figure	Page
Nos.	Nos.
Figure	Page
Nos.	Nos.

Topic 6.3 Steel/Metal

6.3.1.	Prestressing Strands for a Box Beam	6.3.2
6.3.2	Steel Cables with Close-up of Cable Cross-Section	6.3.3
6.3.3	Steel Plate Welded to Girder	6.3.3
6.3.4	Welded I-Girder	6.3.4
6.3.5	Steel Reinforcement Bars	6.3.4
6.3.6	Steel Eyebar	6.3.5
6.3.7	Rolled Beams	6.3.5
6.3.8	Built-up Girder	6.3.6
6.3.9	High Performance Steel Bridge	6.3.8
6.3.10	Steel Corrosion and Complete Section Loss on	
	Girder Webs	6.3.9
6.3.11	Fatigue Crack	6.3.10
6.3.12	Distortion Induced Fatigue	6.3.11
6.3.13	Collision Damage on a Steel Bridge	6.3.12
6.3.14	Heat Damage	6.3.13
6.3.15	Paint Wrinkling	6.3.14
6.3.16	Rust Undercutting at Scratched Area	6.3.14
6.3.17	Pinpoint Rusting	6.3.15
6.3.18	Paint Peeling from Steel Bridge Members	6.3.15
6.3.19	Mudcracking Paint	6.3.15
6.3.20	Corrosion of Steel	6.3.23
6.3.21	Fatigue Crack	6.3.23
6.3.22	Paint Failure on Edge of Steel Truss Member	6.3.24
6.3.23	Water and Salt Runoff Near Expansion Joint	6.3.25
6.3.24	Corroding Rivet Head	6.3.25
6.3.25	Roadway Spray Zone Deficiency	6.3.26
6.3.26	Color of Oxide Film is Critical in the Inspection of	
	Weathering Steel; Dark Black Color in an	
	Indication of Non-protective Oxide	6.3.27
6.3.27	Yellow Orange – Early Development of the Oxide	
	Film (Patina)	6.3.27
6.3.28	Light Brown – Early Development of the Oxide	
	Film (Patina)	6.3.28
6.3.29	Chocolate Brown to Purple Brown - Fully	
	Developed Oxide Film	6.3.28
6.3.30	Black – Non-protective Oxide	6.3.29
6.3.31	Correlation Between Weathering Steel Texture and	
	Condition	6.3.30

Figure Nos.

Page Nos.

Topic 6.4 Fatigue and Fracture in Steel

6.4.1	Silver Bridge Collapse	6.4.1
6.4.2	Mianus River Bridge Collapse	6.4.2
6.4.3	I-35W Mississippi River Bridge Collapse	6.4.2
6.4.4	Load Path Redundant Multi-Girder Bridge	6.4.4
6.4.5	Structurally Redundant Continuous Span Bridge	6.4.5
6.4.6	Internally Redundant Riveted I-Beam	6.4.6
6.4.7	Internally Redundant Riveted Box Shapes	6.4.6
6.4.8	Patch Plate Welded on Riveted Girder Web along	
	Flange Angle	6.4.7
6.4.9	Nonredundant Two-Girder	6.4.8
6.4.10	Brittle Fracture of Cast Iron Specimen	6.4.9
6.4.11	Ductile Fracture of Cold Rolled Steel Specimen	6.4.10
6.4.12	Charpy V-notch Testing Machine	6.4.11
6.4.13	Groove Weld Nomenclature	6.4.13
6.4.14	Fillet Weld Nomenclature	6.4.13
6.4.15	Plug Weld Schematic	6.4.14
6.4.16	Tack Weld	6.4.15
6.4.17	Types of Welded Joints	6.4.16
6.4.18	Exposed Lamination in Steel Slab	6.4.16
6.4.19	Shrinkage Cavity in Steel Billet	6.4.17
6.4.20	Incomplete Penetration of a Double V-Groove Weld.	6.4.18
6.4.21	Crack Initiation from Lack of Fusion in Heat	
	Affected Zone of Electroslag Groove Weld of a Butt	
	Joint	6.4.18
6.4.22	Web to Flange Crack due to Fillet Weld Slag	
	Inclusion	6.4.19
6.4.23	Crack Initiation from Porosity in Longitudinal Web-	
	to-Flange Fillet Weld of Plate Girder	6.4.19
6.4.24	Crack Resulting from Plug Welded Holes	6.4.20
6.4.25	Undercut of a Fillet Weld	6.4.21
6.4.26	Overlap of a Fillet Weld	6.4.21
6.4.27	Incomplete Penetration of a V-Groove Weld	6.4.22
6.4.28	Crack Initiation at Coped Web in Stringer to	
	Floorbeam	6.4.23
6.4.29	Insufficiently Ground Flame Cut of Gusset Plate	
	for Arch to Tie Girder Connection	6.4.23
6.4.30	Thick plate with Two Plates Welded to it and	
	Showing a Lamellar Tear	6.4.24
6.4.31	Severe Collision Damage on a Fascia Girder	6.4.25

Figure Nos.		Page Nos.
6.4.32	Applied Tensile and Compressive Stress Cycles	6.4.26
6.4.33	Part-Through Crack at a Cover Plated Flange	6.4.27
6.4.34	Part-Through Crack Growth at Cover Plate Welded	
	to Flange	6.4.28
6.4.35	Through Crack Growth at Cover Plate Welded to	
	Flange	6.4.29
6.4.36	Through Crack at a Cover Plated Flange	6.4.29
6.4.37	Through Crack has Propagated into the Web	6.4.30
6.4.38	Brittle Fracture – Herringbone Pattern	6.4.30
6.4.39	Crack Growth at Transverse Stiffener Welded to	
	Web	6.4.31
6.4.40	Part-Through Web Crack	6.4.32
6.4.41	Through Crack in the Web	6.4.32
6.4.42	Through-Crack Ready to Propagate into the Flange	6.4.33
6.4.43	AASHTO LRFD Bridge Design Specifications, 5 th	
	Edition, with 2010 Interim Revisions, Table	
	6.6.1.2.3-1 - Detail Categories for Load-Induced	
	Fatigue	6.4.34
6.4.43	AASHTO LRFD Bridge Design Specifications, 5 th	
	Edition, with 2010 Interim Revisions, Table	
	6.6.1.2.3-1 - Detail Categories for Load-Induced	
	Fatigue, continued	6.4.35
6.4.43	AASHTO LRFD Bridge Design Specifications, 5 th	
	Edition, with 2010 Interim Revisions, Table	
	6.6.1.2.3-1 - Detail Categories for Load-Induced	
	Fatigue, continued	6.4.36
6.4.43	AASHTO LRFD Bridge Design Specifications, 5 th	
	Edition, with 2010 Interim Revisions, Table	
	6.6.1.2.3-1 - Detail Categories for Load-Induced	
	Fatigue, continued	6.4.37
6.4.43	AASHTO LRFD Bridge Design Specifications, 5 th	
	Edition, with 2010 Interim Revisions, Table	
	6.6.1.2.3-1 - Detail Categories for Load-Induced	
	Fatigue, continued	6.4.38
6.4.43	AASHTO LRFD Bridge Design Specifications, 5 th	
	Edition, with 2010 Interim Revisions, Table	
	6.6.1.2.3-1 - Detail Categories for Load-Induced	
	Fatigue, continued	6.4.39
6.4.43	AASHTO LRFD Bridge Design Specifications. 5 th	
	Edition, with 2010 Interim Revisions. Table	
	6.6.1.2.3-1 - Detail Categories for Load-Induced	
	Fatigue, continued	6.4.40
	<i>, ,</i>	

Figure Page Nos. Nos. AASHTO LRFD Bridge Design Specifications, 5th 6.4.43 Edition, with 2010 Interim Revisions, Table 6.6.1.2.3-1 - Detail Categories for Load-Induced Fatigue, continued...... 6.4.41 6.4.43 AASHTO LRFD Bridge Design Specifications, 5th Edition, with 2010 Interim Revisions, Table 6.6.1.2.3-1 - Detail Categories for Load-Induced Fatigue, continued...... 6.4.42 Riveted Gusset Plate Connection 6.4.44 6.4.44 6.4.45 6.4.46 Local Triaxial Constraint Condition Resulting in 6.4.47 Fracture on the Hoan Bridge, Milwaukee, Wisconsin 6.4.49 Potential Crack Formation due to Intersecting Welds 6.4.50 6.4.48 Potential Crack Formation for Various Cover Plate 6.4.49 6.4.50 Potential Crack Formation for Cantilever Suspended Span (Potential Cracks Shown in Red)...... 6.4.52 6.4.51 Potential Crack Formation in Vertical Web Weld at 6.4.52 Potential Crack Formation in Vertical Web Weld at Field Welds Perpendicular to Bending Stresses are 6.4.53 Intermittent or Stitch Welded Transverse Stiffeners... 6.4.55 6.4.54 6.4.55 Out-of-Plane Distortion in Web Gap at Diaphragm Web Crack due to Out-of-Plane Distortion at Top 6.4.56 6.4.57 Web Crack due to Out-of-Plane Distortion at Skewed Bridge Producing Out-of-Plane Bending 6.4.58 due to Differential Deflection of Floorbeams and 6.4.59 Lateral Bracing Deflections Producing Out-of-Plane 6.4.60 Cantilevered Floorbeam Producing Out-of-Plane Bending due to Differential Deflection of Stringer 6.4.61 6.4.61 Cracks at Top of Floorbeam Connection to Girder..... 6.4.61 6.4.62 Back-Up Bars Tack Welded to Web and Flange Potentially Producing Abrupt Stress Reversal and 6.4.63 Welded Attachment in Tension Zone of a Beam 6.4.64 6.4.64

	Figure Nos.		Page Nos.
	6.4.65	Flange Termination	6.4.64
	6.4.66	Fracture of a Coped Member	6.4.65
	6.4.67	Blocked Floorbeam Flange	6.4.66
	6.4.68	Cracks Perpendicular or Parallel to Applied Stress	6.4.67
Topic 6.5 Stone Masonry			
v	6.5.1	Stone Masonry Arch	6.5.1
	6.5.2	Splitting in Stone Masonry	6.5.2
Topic 6.6 Fiber Reinforced Polymer (FRP)			
i olymer (i Ki)	661	Concrete Beam Renaired Using FRP	661
	662	Seismic Retrofit of Concrete Columns Using FRP	0.0.1
	0.0.2	Composites	662
	663	CFRP Post-tensioned Steel Girder	663
	664	Externally Bonded CERP Plates to Steel Girder	0.0.5
	0.0.1	Bottom Flange	664
	665	CFRP Plate and GFRP Reinforcing Bars	665
	666	Steel I-Beam (back) and Pultruded FRP I-Beam	0.0.5
	0.0.0	(front)	666
	6.6.7	Pultruded FRP Double-Web Beam	6.6.6
	668	Spools of Continuous Roving	668
	669	Discontinuous Roving	668
	6.6.10	Woven Roving Fabric	6.6.9
	6.6.11	Discontinuous Roving Mat Fabric	6.6.9
	6.6.12	Non-Crimp Fabric	6.6.10
	6.6.13	Fiber Reinforced Concrete (FRC)	6.6.13
	6.6.14	Blistering on a Laminated Surface	6.6.14
	6.6.15	Voids Resulting in Surface Cracks	6.6.15
	6.6.16	Wrinkling of Fabric in Laminated Facesheet	6.6.16
	6.6.17	Fiber Exposure from Improper Handling and	
		Erection Methods	6.6.16
	6.6.18	Scratches on FRP Surface	6.6.17
	6.6.19	Cracks and Discoloration Around Punched Area	6.6.18
	6.6.20	Electronic Tap Testing Device	6.6.20
	6.6.21	Thermographic Image of Bridge Deck	6.6.21
	6.6.21		6.6.01

Topic 7.1 Timber Decks

7.1.1	Plank Deck	7.1.2
7.1.2	Section of a Nailed Laminated Deck	7.1.2

Figure Nos.		Page Nos.
7.1.3	Glue Laminated Deck Panels	7.1.3
7.1.4	Stress-laminated Deck	7.1.4
7.1.5	Structural Composite Lumber Deck using Box	
	Sections	7.1.5
7.1.6	Timber Wearing Surface on a Timber Plank Deck	7.1.6
7.1.7	Inspector Probing Timber with a Pick at Reflective	
	Cracks in the Asphalt Wearing Surface	7.1.8
7.1.8	Wear and Weathering on a Timber Deck	7.1.10
7.1.9	Bearing and Shear Area on a Timber Deck	7.1.10
7.1.10	Edge of Deck Exposed to Drainage, Resulting in	
	Plant Growth	7.1.11
7.1.11	Broken Prestressing Anchors	7.1.11

Topic 7.2 Concrete Decks

7.2.1	CIP concrete Deck with Stay-in-Place Forms	7.2.2
7.2.2	Precast Deck Panels (with Lifting Lugs Evident and	
	Top Beam Flange Exposed)	7.2.3
7.2.3	Shear Connectors Welded to the Top Flange of a	
	Steel Girder for Composite Deck	7.2.4
7.2.4	Prestressed Concrete Beams with Shear Connectors	
	Protruding	7.2.4
7.2.5	Spall Showing Deck Reinforcing Steel	
	Perpendicular to Traffic	7.2.6
7.2.6	Cathodic Protection: Deck Wires Connected to	
	Direct Current Rectifier	7.2.10
7.2.7	Sounding for Delaminated Areas of Concrete	7.2.12
7.2.8	Underside View of Longitudinal Deck Crack	7.2.15
7.2.9	Deteriorated Stay-in-Place Form	7.2.15

Topic 7.3 Fiber Reinforced Polymer (FRP) Decks

7.3.1	Fiber Reinforced Polymer (FRP) Deck	7.3.1
7.3.2	Honeycomb sandwich configuration (Photograph	
	from NCHRP Report 564-field Inspection of In-	
	Service FRP Bridge Decks)	7.3.2
7.3.3	Solid core sandwich configuration (Photograph from	
	NCHRP Report 564 – Field Inspection of In-Service	
	FRP Bridge Decks)	7.3.2

Figure Nos.		Page Nos.
7.3.4	Hollow core sandwich configuration (Photograph	
	from NCHRP Report 564 – Field Inspection of In-	
	Service FRP Bridge Decks)	7.3.2
7.3.5	Use of Truck for Visual Inspection of FRP Deck	7.3.3
7.3.6	Electronic Tap Testing Device	7.3.4
7.3.7	Deck expansion joint	7.3.6
7.3.8	FRP Deck Underside near Superstructure Beam	7.3.6
7.3.9	Clip-type Connection between FRP Deck and Steel	
	Superstructure	7.3.7
7.3.10	Condition rating of FRP deck structure (Source:	
	NCHRP Report 564: Field Inspection of In-Service	
	FRP Bridge Decks: Inspection Manual:	
	Table 7.1.2-1)	7.3.9

Topic 7.4 Steel Decks

7.4.1	Orthotropic Bridge Deck	7.4.1
7.4.2	Underside View of Buckle Plate Deck	7.4.2
7.4.3	Corrugated Steel Floor	7.4.2
7.4.4	Various Patterns of Welded Steel Grid Decks	7.4.3
7.4.5	Riveted Grate Deck	7.4.4
7.4.6	Steel Grid Deck with Slotted Holes (to eliminate	
	welding and riveting)	7.4.5
7.4.7	Concrete-Filled Grid Deck	7.4.6
7.4.8	Filled and Un-filled Steel Grid Deck	7.4.6
7.4.9	Schematic of Exodermic Composite Profile	7.4.7
7.4.10	Broken Members of an Open Steel Grid Deck	7.4.11

Topic 7.5 Deck Joints, Drainage Systems, Lighting and Signs

7.5.1	Strip Seal (Drawing Courtesy of the D.S. Brown Co.	7.5.2
7.5.2	Pourable Joint Seal	7.5.3
7.5.3	Cross Section of a Pourable Joint Seal	7.5.3
7.5.4	Compression Joint Seal with Steel Angle Armoring	7.5.4
7.5.5	Cross Section of a Compression Joint Seal with	
	Steel Angle Armoring	7.5.4
7.5.6	Cross Section of a Cellular Seal	7.5.5
7.5.7	Modular Seal	7.5.5
7.5.8	Schematic Cross Section of a Modular Seal	7.5.6
7.5.9	Plank Seal	7.5.6

Figure		Page
Nos.		Nos.
7.5.10	Sheet Seal	7.5.7
7.5.11	Asphaltic Expansion Joint	7.5.8
7.5.12	Open Expansion Joint	7.5.8
7.5.13	Cross Section of a Open Expansion Joint	7.5.9
7.5.14	Finger Plate Joint	7.5.10
7.5.15	Cross Section of a Cantilever Finger Plate Joint	7.5.10
7.5.16	Supported Finger Plate Joint	7.5.11
7.5.17	Sliding Plate Joint	7.5.12
7.5.18	Cross Section of a Sliding Plate Joint	7.5.12
7.5.19	Bridge Deck Scupper (left) and Deck Drain (right)	7.5.14
7.5.20	Outlet Pipe	7.5.15
7.5.21	Downspout Pipe	7.5.15
7.5.22	Drainage Trough	7.5.15
7.5.23	Weep Holes	7.5.16
7.5.24	Debris Lodged in a Sliding Plate Joint	7.5.20
7.5.25	Dirt in a Compression Seal Joint	7.5.20
7.5.26	Improper Vertical Alignment at a Finger Plate Joint	7.5.21
7.5.27	Failed Compression Seal	7.5.22
7.5.28	Asphalt Wearing Surface over an Expansion Joint	7.5.22
7.5.29	Support System under a Finger Plate Joint	7.5.23
7.5.30	Clogged Scupper	7.5.25
7.5.31	Outlet Pipe with Cleanout Plugs	7.5.26
7.5.32	Drainage Trough with Debris Accumulation	7.5.27
7.5.33	Sign and Light Structures Attached to a Bridge	7.5.28
7.5.34	Sign Attachment Exhibiting Anchor Pullout	7.5.28
7.5.35	Sign Mount with Loose Adhesive Anchorage	7.5.29

Topic 7.6 Safety Features

7.6.1	Bridge Safety Feature	7.6.1
7.6.2	Traffic Safety Features	7.6.3
7.6.3	Bridge Railing, Transition, Approach Guardrail and	
	Approach Guardrail End	7.6.3
7.6.4	2010 AASHTO LRFD Bridge Specifications Test	
	Level Index (based on the NCHRP Report 350 Test	
	Level Index)	7.6.7
7.6.5	2009 AASHTO Manual for Assessment of Safety	
	Hardware (MASH) Test Level Index	7.6.7
7.6.6	Acceptable Bridge Rail	7.6.11
7.6.7	Bridge Rail Guide	7.6.11
7.6.8	Acceptable Transition	7.6.12
7.6.9	Approach Guardrail System and Approved Plastic	
	Offset Block	7.6.13
Figure Nos.		Page Nos.
----------------	--	--------------
7.6.10	W-Shaped Guardrail End Flared and Buried into an	
	Embankment	7.6.14
7.6.11	Unacceptable Blunt Ends	7.6.15
7.6.12	Deficiency Steel Post Bridge Railing	7.6.17
7.6.13	Approach Guardrail Collision Damage	7.6.18
7.6.14	Erosion Reducing Post Embedment	7.6.18
7.6.15	Proper Nesting of Guardrail at Transition	7.6.19
7.6.16	Impact Attenuator	7.6.20
7.6.17	Timber Traffic Safety Features, Rocky Mountain	
	National Park	7.6.20

Topic 8.1 Solid Sawn Timber Bridges

8.1.1	Elevation View of a Solid Sawn Multi-Beam Bridge.	8.1.1
8.1.2	Underside View of a Solid Sawn Multi-Beam	
	Bridge	8.1.2
8.1.3	Elevation View of Covered Bridge	8.1.3
8.1.4	Inside View of Covered Bridge Showing King Post	
	Truss Design	8.1.4
8.1.5	Town Truss Covered Bridge	8.1.5
8.1.6	Common Covered Bridge Trusses	8.1.5
8.1.7	Schematic of Burr Arch-truss Covered Bridge	8.1.6
8.1.8	Burr Arch-truss Covered Bridge	8.1.6
8.1.9	Inside View of Covered Bridge with Burr Arch-truss	
	Design	8.1.7
8.1.10	Town Truss Design	8.1.7
8.1.11	Bearing Area of Typical Solid Sawn Beam	8.1.9
8.1.12	Horizontal Shear Crack in a Timber Beam	8.1.10
8.1.13	Decay in a Timber Beam	8.1.11
8.1.14	Typical Timber End Diaphragm	8.1.12

Topic 8.2 Glulam Timber Bridges

8.2.1	Elevation View of a Glulam Multi-beam Bridge	8.2.1
8.2.2	Underside View of a Glulam Multi-beam Bridge	8.2.2
8.2.3	Timber Through Truss Typical Section	8.2.3
8.2.4	Bowstring Truss Pedestrian Bridge	8.2.4
8.2.5	Parallel Chord Truss Pedestrian Bridge (Eagle	
	River, Alaska)	8.2.4
8.2.6	Glulam Arch Bridge over Glulam Multi-beam	
	Bridge (Keystone Wye Interchange, South Dakota)	8.2.5
8.2.7	Glulam Arch Bridge (West Virginia)	8.2.5

Figure		Page
Nos.		Nos.
8.2.8	Typical Glulam Diaphragm	8.2.6
8.2.9	Bearing Area of Typical Glulam Beam	8.2.8
8.2.10	Close-up View of Glulam Bridge Showing	
	Laminations	8.2.9
8.2.11	Elevation View of Beam of Glulam Multi-beam	
	Bridge	8.2.9
8.2.12	Decay on Glulam Bridge	8.2.10
8.2.13	Typical Diaphragm for a Glulam Multi-beam Bridge	8.2.11
8.2.14	Glulam Beams with Numerous Fastener Locations	8.2.11

Topic 8.3 Stress-Laminated Timber Bridges

8.3.1	Stress-Laminated Timber Slab Bridge Carrying a	
	90,000-Pound Logging Truck (Source: Barry	
	Dickson, West Virginia University)	8.3.1
8.3.2	Typical Section of Stress-Laminated Timber Slab	
	Bridge	8.3.2
8.3.3	Stress-Laminated Timber Slab Bridge	8.3.2
8.3.4	Glulam Stress-Laminated Timber Slab Bridge	8.3.3
8.3.5	Typical Section of Stress-Laminated Timber Tee	
	Beam Bridge (Source: Barry Dickson, West Virginia	
	University)	8.3.3
8.3.6	Elevation View of Stress-Laminated Timber Tee	
	Beam Bridge (West Virginia)	8.3.4
8.3.7	Typical Section of Stress-Laminated Timber Box	
	Beam (Source: Barry Dickson, West Virginia	
	University)	8.3.4
8.3.8	Stress-Laminated Timber Box Beam Bridge Being	
	Erected	8.3.5
8.3.9	Stress-Laminated Timber K-Frame Bridge	8.3.5
8.3.10	Broken Stressing Rods	8.3.7
8.3.11	Close-up View of End of a Stress-Laminated Timber	
	Bridge Showing Laminations	8.3.8
	5 5	

Topic 9.1 Cast-in-Place Slabs

9.1.1	Typical Simple Span Cast-in-Place Slab Bridge	9.1.1
9.1.2	Typical Multi-Span Cast-in-Place Slab Bridge	9.1.2
9.1.3	Steel Reinforcement in a Simply Supported	
	Concrete Slab	9.1.3
9.1.4	Bearing Area: Cast-in-Place Slab	9.1.6

Figure		Page
Nos.		Nos.
9.1.5	Diagonal Shear Cracks Close to the Ends of a Slab	
	Bridge	9.1.6
9.1.6	Shear Zone on the Underside of a Continuous Slab	
	Bridge Near a Pier	9.1.7
9.1.7	Inspector Examining and Documenting Deficiencies	
	in Concrete Slab	9.1.8
9.1.8	Concrete Slab Tension Zone: Delamination,	
	Efflorescence, Rust and Stains	9.1.7
9.1.9	Deteriorated Slab Fascia due to Roadway Deicing	
	Agents	9.1.9

Topic 9.2 Cast-In-Place Tee Beams

9.2.1	Simple Span Tee Beam Bridge	9.2.1
9.2.2	Typical Tee Beam Cross Section	9.2.2
9.2.3	Typical Tee Beam Layout	9.2.2
9.2.4	Comparison Between Tee Beam and Concrete	
	Encased Steel I-beam	9.2.3
9.2.5	Concrete Encased Steel I-beam	9.2.3
9.2.6	Tee Beam Primary and Secondary Members	9.2.4
9.2.7	Steel Reinforcement in a Concrete Tee Beam	9.2.5
9.2.8	Bearing Area of Typical Cast-in-Place Concrete Tee	
	Beam Bridge	9.2.7
9.2.9	Spalled Tee Beam End	9.2.8
9.2.10	Deteriorated Tee Beam Bearing Area	9.2.8
9.2.11	Steel Bearing Supporting a Cast-in-Place Concrete	
	Tee Beam	9.2.9
9.2.12	Shear Zone of Cast-in-Place Concrete Tee Beam	
	Bridge	9.2.9
9.2.13	Flexure Cracks on a Tee Beam Stem	9.2.10
9.2.14	Flexure Cracks in Tee Beam Flange/Deck	9.2.10
9.2.15	Stem of a Cast-in-Place Concrete Tee Beam with	
	Cracking and Efflorescence	9.2.11
9.2.16	Spall on the Bottom of the Stem of a Cast-in-Place	
	Tee Beam with Corroded Main Steel Exposed	9.2.12
9.2.17	Asphalt Covered Tee Beam Deck	9.2.13
9.2.18	Deteriorated Tee Beam Stem Adjacent to Drain	
	Hole	9.2.13
9.2.19	Deteriorated Tee Beam End Due to Drainage	9.2.14
9.2.20	Collision damage to Tee Beam Bridge Over a	
	Highway	9.2.14
9.2.21	Components/Elements for Evaluation	9.2.15

FigurePageNos.Nos.

Topic 9.3 Conventionally Reinforced Concrete Two-Girder System

9.3.1	Concrete Deck Two-Girder Bridge	9.3.1
9.3.2	Concrete Through Two-Girder Bridge	9.3.2
9.3.3	Concrete Deck Two-Girder, Underside View	9.3.2
9.3.4	Concrete Through Two-Girder Elevation View	9.3.3
9.3.5	Steel Reinforcement in a Concrete Through Two-	
	Girder	9.3.4
9.3.6	Bearing Area of a Through Two-Girder Bridge	9.3.6
9.3.7	Typical Elevation View of a Through Two-Girder	
	Bridge with Tension Zones Indicated	9.3.7
9.3.8	Exposed Reinforcement in a Through Two-Girder	
	(under hammer)	9.3.8
9.3.9	Close-up of an Interior Face of a Through Two-	
	Girder with Heavy Scaling Due to Deicing Agents	9.3.9

Topic 9.4 Concrete Channel Beams

9.4.1 9.4.2	Underside View of Precast Channel Beam Bridge Underside View of a Cast-in-Place Channel Beam	9.4.1
	Bridge	9.4.1
9.4.3	General View of a Precast Channel Beam Bridge	9.4.2
9.4.4	Cross Section of a Typical Channel Beam	9.4.3
9.4.5	Joint Leakage Between Channel Beams	9.4.7
9.4.6	Top of Deck View of Precast Channel Beam Bridge .	9.4.8
9.4.7	Stem Tie-Bolts	9.4.8
9.4.8	Close-up of Stem Tie-Bolts	9.4.9
9.4.9	Close-up of Intermediate Diaphragm	9.4.9
9.4.10	Components/Elements for Evaluation	9.4.11

Topic 9.5 Concrete Arches

9.5.1	Open Spandrel Arch Bridge	7.5.1
9.5.2	Multi-Span Closed Spandrel Arch Bridge	7.5.2
9.5.3	Concrete Through Arch Bridge	7.5.3
9.5.4	Precast Concrete Arch with Integral Vertical Legs	7.5.4
9.5.5	Precast Segmental Concrete Arch	7.5.4

Figure Nos.		Page Nos.
9.5.6	Precast Post-tensioned Concrete Arch without	
	Spandrel Columns	7.5.5
9.5.7	Primary and Secondary Members of an Open	
	Spandrel Arch	9.5.6
9.5.8	Primary Members of a Closed Spandrel Arch	9.5.7
9.5.9	Open Spandrel Arch and Spandrel Bent Column	
	Reinforcement	9.5.8
9.5.10	Spandrel Bent Cap Reinforcement	9.5.8
9.5.11	Reinforcement in a Closed Spandrel Arch	9.5.9
9.5.12	Arch/Skewback Interface	9.5.12
9.5.13	Spandrel Column Bent Cap Interface	9.5.12
9.5.14	Spandrel Bent Tension Zones	9.5.13
9.5.15	Deteriorated Arch/Spandrel Wall Interface	9.5.14
9.5.16	Severe Scaling and Spalling on a Spandrel Column	9.5.14
9.5.17	Inspection and Documentation of Arch Strut	
	Deficiencies	9.5.15
9.5.18	Scaling and Contamination on an Arch Rib Due to a	
	Failed Drainage System	9.5.15
9.5.19	Measurements for Open and Closed Spandrel	
	Arches	9.5.18

Topic 9.6 Concrete Rigid Frames

061	Multi anon Congrete Digid Frame Dridges	061
9.0.1	Sin 1 Deter 1 Concrete Rigid France Bridges	9.0.1
9.6.2	Single-span Rectangular Concrete Rigid Frame	
	Bridge	9.6.2
9.6.3	Three Span Concrete K-frame Bridge	9.6.2
9.6.4	Elevation of a Single Frame	9.6.3
9.6.5	Elevation of a K-frame	9.6.3
9.6.6	Deflected Simply Supported Slab versus Deflected	9.6.4
	Frame Shape	
9.6.7	Primary Reinforcement in a Single Span Frame	9.6.4
9.6.8	Primary Reinforcement in a Multi-span Slab or	
	Beam Frame	9.6.5
9.6.9	Primary Reinforcement in a K-frame	9.6.5
9.6.10	Shear Zones in Single Span and Multi-span Frames	9.6.8
9.6.11	Tension and Compression Zones in a Single Span	
	Frame	9.6.9
9.6.12	Tension and Compression Zones in a Multi-span	
	Frame	9.6.9
9.6.13	Roadway of a Rigid Frame Bridge with Asphalt	
	Wearing Surface	9.6.10
9.6.14	Longitudinal Joint Between Slab Beam Frames	9.6.10

Figure Nos.	Page Nos.
Topic 9.7 Precast and Prestressed Slabs	
9.7.1 Typical Prestressed Slab Beam Bridge	9.7.1
9.7.2 Cross Section of a Typical Voided Slab	9.7.2
973 Poutre Dalle Precast Slab Bridge	973
974 Poutre Dalle Bridge Schematic	973
975 Slab Beam Bridge Tension and Shear Reinforcement	974
9.7.6 Leaking Joint between Adjacent Slab Units	977
9.7.7 Exposed Strands in a Prestressed Slab Beam	9.7.8
Topic 9.8	
Prestressed	
Double Tees	
9.8.1 Typical Prestressed Double Tee Beam	9.8.1
9.8.2 Prestressed Double Tee Beam Typical Section	9.8.2
9.8.3 Dapped End of a Prestressed Double Tee Beam	9.8.2
9.8.4 Steel Reinforcement in a Prestressed Double Tee	
Beam	9.8.4
9.8.5 Crack Locations for Dapped End Double Tee Beams	9.8.6
9.8.6 Components/Elements for Evaluation	9.8.10
Topic 9.9 Prestressed I-Beams and Bulb-Tees	
9.9.1 Prestressed I-beam Superstructure	9.9.1
9.9.2 AASHTO Cross Sections of Prestressed I-beams	9.9.2
9.9.3 AASHTO Prestressed I-beam Bridge	9.9.2
9.9.4 Cross Section of AASHTO-PCI Bulb-Tee Beams	9.9.3
9.9.5 Placement of an AASHTO-PCI Bulb-Tee Beam	9.9.3
9.9.6 Cross Section of AASHTO-PCI Bulb-Tee Beams	9.9.4
9.9.7 Reactive Powder Concrete (RPC) Prestressed	
X-beam	9.9.5
9.9.8 Continuous Prestressed I-beam Schematic	9.9.6
9.9.9 Continuous Prestressed I-beam Bridge	9.9.6
9 9 10 Spliced Bulb-Tees with Haunched Girder Sections	
Over Piers	9.9.7
9.9.11 Extended Stirrups to Obtain Composite Action	9.9.7
9.9.12 Concrete Intermediate and End Dianhragms	9.9.8
9.9.13 Prestressed I-beam Reinforcement	999
9.9.14 Prestressed Bulb-Tee Beam Reinforcement	9.9.9

Figure Nos		Page Nos
9.9.15	Bearing Area of a Typical Prestressed I-beam	9.9.12
9.9.16	Spalling Due to Poor Concrete Placement	9.9.12
9.9.17	Flexure Crack	9.9.14
9.9.18	Concrete End Diaphragm	9.9.15
9.9.19	Leakage of Water at Joint between Spans	9.9.15
9.9.20	Inspectors Evaluating Collision Damage on	
	Prestressed Concrete I-beam	9.9.16
9.9.21	Collision Damage Repair on Prestressed Concrete	
	I-beam: Note Epoxy Injection Ports and Gunite	
	Repair	9.9.17

Topic 9.10 Prestressed Box Beams

9.10.1	Typical Box Beam Bridge	9.10.1
9.10.2	Box Beam Cross Section	9.10.2
9.10.3	Box Beams at Fabrication Plant Showing Stirrups	
	Extended as Shear Connectors and Extended	
	Reinforcement for Continuity	9.10.3
9.10.4	Prestressed Box Beam Cross Sections: Adjacent and	
	Spread Box Beams	9.10.4
9.10.5	Adjacent Box Beams: Top Flanges Acting as the	
	Deck	9.10.5
9.10.6	Transverse Post-tensioning of an Adjacent Box	
	Beam Bridge	9.10.6
9.10.7	Underside of a Typical Spread Box Beam	9.10.7
9.10.8	End and Intermediate Diaphragms on a Spread Box	
	Beam Bridge	9.10.8
9.10.9	Schematic of Internal Diaphragms	9.10.8
9.10.10	Typical Prestressed Box Beam Reinforcement	9.10.9
9.10.11	Spalled Beam Ends with Exposed Prestressing	
	Reinforcement	9.10.12
9.10.12	Exposed Shear Reinforcement at End of Box Beam	9.10.12
9.10.13	Longitudinal Cracks in Bottom Flange at Beam	9.10.13
9.10.14	Diagonal Shear Crack in Web of Beam	9.10.13
9.10.15	Spall and Exposed/Corroded Reinforcement	9.10.15
9.10.16	Close-up of Failed Strands due to Corrosion	9.10.15
9.10.17	Joint Leakage and Rust Stain	9.10.17
9.10.18	Close-up of Box Beam Collision Damage	9.10.17
9.10.19	Components/Elements for Evaluation	9.10.20
9.10.20	View Northeast of I-70 EB from Beneath Span 3	9.10.20
9.10.21	View East (Ahead Segments) from SR1014 Above	
	Pier 2	9.10.21
9.10.22	Post-Collapse Material Testing Assessment	9.10.22

9.10.23Post-Collapse Prestressing Strand Wire Fracture Laboratory Assessment	 9.10.23 Post-Collapse Prestressing Strand Wire Fracture Laboratory Assessment 9.10.24 Prestressing Strand Corrosion and Section Loss 	9.10.23 9.10.24
 Laboratory Assessment	Laboratory Assessment9.10.24 Prestressing Strand Corrosion and Section Loss	9.10.239.10.24
 9.10.24 Prestressing Strand Corrosion and Section Loss Caused by Moisture Through Longitudinal Cracking 9.10.25 Prestressing Strand Corrosion and Section Loss Caused by Moisture Through Longitudinal Cracking and Shear Reinforcement Bars Transferring Moisture to Adjacent Longitudinal Reinforcement 9.10.26 Longitudinal Cracks (Top) and Corresponding 9.10.27 Laboratory Testing of Torsion-Shear Cracking Near Barrier Joints 9.10.25 	9.10.24 Prestressing Strand Corrosion and Section Loss	9.10.24
 Caused by Moisture Through Longitudinal Cracking 9.10.24 9.10.25 Prestressing Strand Corrosion and Section Loss Caused by Moisture Through Longitudinal Cracking and Shear Reinforcement Bars Transferring Moisture to Adjacent Longitudinal Reinforcement 9.10.24 9.10.26 Longitudinal Cracks (Top) and Corresponding 9.10.24 Corroded Prestressing Strands after Concrete Removed (Bottom) 9.10.27 Laboratory Testing of Torsion-Shear Cracking Near Barrier Joints	Θ	9.10.24
 9.10.25 Prestressing Strand Corrosion and Section Loss Caused by Moisture Through Longitudinal Cracking and Shear Reinforcement Bars Transferring Moisture to Adjacent Longitudinal Reinforcement 9.10.24 9.10.26 Longitudinal Cracks (Top) and Corresponding 9.10.24 Corroded Prestressing Strands after Concrete Removed (Bottom)	Caused by Moisture Through Longitudinal Cracking	
Caused by Moisture Through Longitudinal Cracking and Shear Reinforcement Bars Transferring Moisture to Adjacent Longitudinal Reinforcement 9.10.24 9.10.26 Longitudinal Cracks (Top) and Corresponding 9.10.24 Corroded Prestressing Strands after Concrete Removed (Bottom) 9.10.27 Laboratory Testing of Torsion-Shear Cracking Near Barrier Joints 9.10.25	9.10.25 Prestressing Strand Corrosion and Section Loss	
 and Shear Reinforcement Bars Transferring Moisture to Adjacent Longitudinal Reinforcement 9.10.24 9.10.26 Longitudinal Cracks (Top) and Corresponding Corroded Prestressing Strands after Concrete Removed (Bottom) 9.10.27 Laboratory Testing of Torsion-Shear Cracking Near Barrier Joints	Caused by Moisture Through Longitudinal Cracking	
 Moisture to Adjacent Longitudinal Reinforcement 9.10.24 9.10.26 Longitudinal Cracks (Top) and Corresponding 9.10.24 Corroded Prestressing Strands after Concrete Removed (Bottom)	and Shear Reinforcement Bars Transferring	
9.10.26Longitudinal Cracks (Top) and Corresponding Corroded Prestressing Strands after Concrete Removed (Bottom)9.10.249.10.27Laboratory Testing of Torsion-Shear Cracking Near Barrier Joints	Moisture to Adjacent Longitudinal Reinforcement	9.10.24
Corroded Prestressing Strands after Concrete Removed (Bottom) 9.10.27 Laboratory Testing of Torsion-Shear Cracking Near Barrier Joints	9.10.26 Longitudinal Cracks (Top) and Corresponding	9.10.24
 Removed (Bottom)	Corroded Prestressing Strands after Concrete	
9.10.27 Laboratory Testing of Torsion-Shear Cracking Near Barrier Joints	Removed (Bottom)	
Barrier Joints 9.10.25	9.10.27 Laboratory Testing of Torsion-Shear Cracking Near	
	Barrier Joints	9.10.25
9.10.28 Cracking Near Barrier Joints 9.10.25	9.10.28 Cracking Near Barrier Joints	9.10.25
9.10.29 Water Leakage Through Parapet Deflection Joints 9.10.26	9.10.29 Water Leakage Through Parapet Deflection Joints	9.10.26
0.10.20 Unforegroup Entriportion Problems 0.10.27	9.10.30 Unforeseen Fabrication Problems	9.10.27

Topic 9.11 Concrete Box Girders

9.11.1	Segmental Precast Concrete Box Girder Bridge	9.11.1
9.11.2	Cast-in-place Concrete Box Girder Bridge	9.11.2
9.11.3	Multi-cell Girder Post Tensioned	9.11.2
9.11.4	Cast-in-place Concrete Box Girder Bridge	9.11.3
9.11.5	High Level Casting Formwork on Falsework	9.11.3
9.11.6	At-Grade Formwork with Post-tensioning Ducts	9.11.4
9.11.7	At-grade Casting – After Supporting Earth Removed	9.11.4
9.11.8	Basic Components/Elements of a Concrete Box	
	Girder	9.11.5
9.11.9	Replaceable Deck on a Multiple Cell Cast-in-place	
	Box Girder	9.11.5
9.11.10	Primary and Secondary Reinforcement in a Concrete	
	Box Girder	9.11.6
9.11.11	Post-tensioning Spiral Anchorage Reinforcement	
	Prior to Concrete Placement	9.11.7
9.11.12	Three-Dimensional Model Illustrating Confinement	
	Reinforcement Around Anchorage and Deviation	
	Blocks (Click to Open Interactive Model)	9.11.7
9.11.13	Adjacent Single Cell Boxes with Closure Pour	9.11.8
9.11.14	Segmental Concrete Bridge	9.11.9
9.11.15	Close-up of Box Girder Segments	9.11.9
9.11.15	Cast-in-place Box Girder Segment	9.11.10
9.11.16	Box Girder Segment	9.11.10
9.11.17	Cast-in-place Box Girder Segment	9.11.11

Figure		Page
Nos.		Nos.
9.11.18	Box Girder Segment During Construction with Temporary and Permanent Post-Tensioning (PT)	9.11.11
	Bars	
9.11.19	Two Balanced Cantilever Methods – Using Cranes	
	with Stability Towers At Each Pier and Using An	
	Overhead Launching Gantry	9.11.12
9.11.20	Balanced Cantilever Construction Using an	
0.11.01	Overhead Launching Gantry	9.11.13
9.11.21	Typical Features of Precast Cantilever Box Girder	0 1 1 1 0
0 11 00	Segments	9.11.13
9.11.22	Span-by-Span Construction (with Erection Truss)	9.11.14
9.11.23	Span-by-Span Construction (with Erection Truss)	9.11.14
9.11.24	Span-by-Span Total Span Erection (Lifting)	9.11.15
9.11.25	Progressive Placement Construction	9.11.16
9.11.26	Incremental Launching Method	9.11.16
9.11.27	Incremental Launching Overview (Note Temporary	0 1 1 17
0 11 20	Pile Bent)	9.11.1/
9.11.28	Bearing Area of a Box Girder Bridge	9.11.20
9.11.29	Box Girder Cracks Induced by Shear	9.11.20
9.11.30	Box Girder Cracks Induced by Direct Tension	9.11.21
9.11.31	Box Girder Cracks induced by Flexure (Positive	0 11 22
0 11 22	Moment)	9.11.22
9.11.32	Box Girder Cracks induced by Flexure (Negative	0 11 22
0 11 22	Moment)	9.11.22
9.11.33	Web Splitting Near on Anabarage Pleak	9.11.23
9.11.34	Deviation Diack Used as a Held Deven Doint for	9.11.24
9.11.55	External Doct Tancianing	0 11 24
0 11 26	Temporary Deviation Placks Used to Maintain	9.11.24
9.11.30	Temporary Deviation Blocks Used to Maintain Tendon Alignment During Construction	0 11 25
0 11 27	Congrete Poy Girder Drein Hole with Sereen	9.11.23
9.11.37	Pay Girder Creaks Induced by Torsion and Shear	9.11.20
0 11 20	Thermally Induced Transverse Cracks in Boy Girder	9.11.27
9.11.39	Flanges	0 11 28
0 11 /0	Thermally Induced Longitudinal Cracks at Change	9.11.20
9.11.40	in Box Girder Cross Section	0 11 28
0 11 /1	Post-Tensioning Tendon Duct	9.11.20
9.11.41 9.11.42	Interior Formwork Left in Place	0 11 30
9.11.42	Location of Observation Points Across the Ton	9.11.30
J.11. - J	Flange	0 11 31
9 11 44	Segmental Box Girder Bearings at Intermediate Pier	9 11 32
0 11 /5	Segmental Box Girder Cracks Adjacent to	7.11.33
J.11.TJ	Anchorage Block	9 11 22
9 11 46	Close-up View of Box Girder Joint	9 11 3/
7.11.40	crose up view of Box Grider Joint	7.11.74

Figure Nos.		Page Nos.
9.11.47	View of Box Girder Joint (Shear Keys) and	
	Deviation Block	9.11.35
9.11.48	Box Girder Interior (End) Diaphragm and Post-	
	Tensioning Ducts	9.11.35
9.11.49	Components/Elements for Evaluation	9.11.38

Topic 10.1 Rolled Steel Multi-Beams and Fabricated Steel Multi-Girders

10.1.1	Simple Span Rolled Multi-Beam Bridge	10.1.2
10.1.2	Continuous Span Rolled Multi-Beam Bridge with	
	Pin & Hanger	10.1.2
10.1.3	Rolled Multi-Beam Bridge with a Cover Plate	10.1.3
10.1.4	Built-up Riveted Plate Girder	10.1.3
10.1.5	Welded Plate Girder	10.1.4
10.1.6	Single Span Fabricated Multi-girder Bridge	10.1.4
10.1.7	Continuous Span Fabricated Multi-girder Bridge	10.1.5
10.1.8	Curved Fabricated Multi-girder Bridge	10.1.5
10.1.9	Fabricated Multi-girder Bridge with Pin & Hanger	
	Connection	10.1.6
10.1.10	Combination Rolled Beams and Fabricated Girders	10.1.6
10.1.11	Web Insert Plate for Multi-beam	10.1.7
10.1.12	Fabricated Variable Depth Girder Bridge	10.1.8
10.1.13	Rolled Beam (Primary Member) with Diaphragm	
	(Secondary Member)	10.1.9
10.1.14	Curved Multi-Girder Bridge	10.1.10
10.1.15	Straight Multi-Girder Bridge	10.1.10
10.1.16	Corroded Shear Zone on a Rolled Multi-beam	
	Bridge	10.1.13
10.1.17	Flexural Zone on a Multi-Span Simple Span Rolled	
	Multi-Beam Bridge	10.1.14
10.1.18	Flexural Zone on a Fabricated Continuous Span	
	Multi-Girder Bridge	10.1.14
10.1.19	Negative Moment Region on a Continuous Span	
	Rolled Multi-Beam Bridge	10.1.15
10.1.20	Negative Moment Region on a Continuous Span	
	Fabricated Multi-Girder Bridge	10.1.15
10.1.21	End Diaphragm	10.1.16
10.1.22	Intermediate Diaphragm	10.1.17
10.1.23	Collision Damage on a Rolled Multi-Beam Bridge	10.1.18

Figure		Page
Nos.		Nos.
10.1.24	Collision Damage on a Fabricated Multi-Girder	
	Bridge	10.1.18

Topic 10.2 Steel Two-Girder Systems

10.2.1	General View of a Dual Deck Girder Bridge	10.2.1
10.2.2	Through Girder Bridge	10.2.2
10.2.3	Through Girder Bridge with Limited	
	Underclearance	10.2.2
10.2.4	Through Girder Bridge with Three Girders	10.23
10.2.5	Two-Girder Bridge with Girder-Floorbeam System	10.2.4
10.2.6	Two-Girder Bridge with Girder-Floorbeam-Stringer	
	System	10.2.4
10.2.7	Two-Girder Bridge with GFS System with Stacked	
	Floorbeam and Stringers	10.2.5
10.2.8	Underside View of Deck Girder Bridge with Lateral	
	Bracing System	10.2.6
10.2.9	Underside View of Through Girder Bridge with	
	Lateral Bracing	10.2.6
10.2.10	Two-Girder Bridge with Pin-and-Hanger Assembly	10.2.7
10.2.11	Shear Zone on a Deck Girder Bridge	10.2.10
10.2.12	Web Area Near Support on a Through Girder Bridge	10.2.10
10.2.13	Flexural Zone on a Two-Girder Bridge	10.2.11
10.2.14	Longitudinal Stiffener in Tension Zone on a Two-	
	Girder Bridge	10.2.11
10.2.15	Flexural Zone on a Through Girder Bridge	10.2.12
10.2.16	Lateral Bracing Connection on a Deck Girder	
	Bridge	10.2.12
10.2.17	Lateral Bracing Connection on a Through Girder	
	Bridge	10.2.13
10.2.18	Collision Damage to a Deck Girder Bridge	10.2.14
10.2.19	Collision Damage to a Through Girder Bridge	10.2.14

Topic 10.3 Steel Box Beams and Girders

10.3.1	Simple Span Box Girder Bridge	10.3.1
10.3.2	Curved Box Girder Bridge	10.3.1
10.3.3	Box Girders with Multiple Interior Webs	10.3.2
10.3.4	Spread Box Girders	10.3.2
10.3.5	Diaphragms – K-Bracing Internal Transverse	
	Stiffeners	10.3.3

Figure Nos.		Page Nos.
10.3.6	External Diaphragm	10.3.4
10.3.7	Box Girder Access Door	10.3.4
10.3.8	Box Girder Cross Section with Composite Deck	10.3.5
10.3.9	Box Girder Cross Section (at floorbeam) with	
	Orthotropic Steel Plate Deck	10.3.5
10.3.10	Box Girder Shear Zone	10.3.8
10.3.11	Continuous Box Girders	10.3.8
10.3.12	Non-Redundant Box Girder Bridges	10.3.11
10.3.13	Redundant Box Girder Bridge	10.3.11

Topic 10.4 Steel Trusses

10.4.1	Single Span Truss	10.4.1
10.4.2	Through-Pony-Deck Truss Comparisons	10.4.2
10.4.3	Through Truss	10.4.2
10.4.4	Pony Truss	10.4.3
10.4.5	Deck Truss	10.4.3
10.4.6	Suspension Bridge with Stiffening Truss	10.4.4
10.4.7	Deck Arch Bridge with Stiffening Truss	10.4.4
10.4.8	Vertical Lift Bridge	10.4.5
10.4.9	Various Truss Designs	10.4.5
10.4.10	Single (Simple) Span Camel Back Pratt Truss	10.4.6
10.4.11	Single (Simple) Span Through Truss	10.4.6
10.4.12	Multiple Span Pony Truss	10.4.7
10.4.13	Multiple Span Through Truss	10.4.7
10.4.14	Continuous Through Truss	10.4.8
10.4.15	Cantilever Deck Truss	10.4.8
10.4.16	Cantilever Through Truss	10.4.9
10.4.17	Pin-and-Hanger Assembly for Cantilevered Truss	10.4.9
10.4.18	Truss Members, Floor systems and Bracing	10.4.10
10.4.19	Rolled Steel Shapes	10.4.10
10.4.20	Built-Up Sections	10.4.11
10.4.21	Axial Loads in Truss Chord Members	10.4.12
10.4.22	"Imaginary Cable – Imaginary Arch"	10.4.13
10.4.23	Vertical Member Stress Prediction Method	10.4.14
10.4.24	Vertical Member Stress Prediction Method	10.4.15
10.4.25	Vertical Member Stress Prediction Method	10.4.15
10.4.26	Truss Panel Point using Shop Rivets and Field Bolts	10.4.16
10.4.27	Pin Connected Truss	10.4.17
10.4.28	Truss Panel Point Numbering System	10.4.17
10.4.29	Deck Truss	10.4.18
10.4.30	A Pennsylvania Truss, a Subdivided Pratt Truss with	
	a Camel Back Top Chord	10.4.18
10.4.31	Floorbeam Stringer Floor System	10.4.19

Figure Nos		Page Nos
10.4.32	Floorbeam Floor System	10.4.20
10.4.33	Inspection of Upper Lateral Bracing	10.4.20
10.4.34	Lower Lateral Bracing	10.4.21
10.4.35	Lateral Bracing Gusset Plate	10.4.21
10.4.36	Sway Bracing	10.4.22
10.4.37	Sway Bracing	10.4.22
10.4.38	Portal Bracing with Attached Load Posting Sign	10.4.23
10.4.39	Pony Truss "Sway Bracing"	10.4.23
10.4.40	Truss Design Drawings: Member Load Table	10.4.27
10.4.41	Corrosion and Section Loss on Truss Bottom Chord	10.4.28
10.4.42	Inside of Box Chord Member	10.4.29
10.4.43	Cracked Forge Zone on an Eyebar	10.4.29
10.4.44	Cracked Forge Zone on a Loop Rod	10.4.30
10.4.45	Bottom Chord with Eyebars	10.4.30
10.4.46	Welded Repair to Loop Rod	10.4.31
10.4.47	Bowed Bottom Chord Eyebar Member	10.4.31
10.4.48	Buckled Bottom Chord Member Due to Abutment	
	Movement	10.4.32
10.4.49	Collision Damage to Truss members Due to	
	Overheight Vehicle	10.4.33
10.4.50	Buckled End Post	10.4.33
10.4.51	Gusset Plate Connection with Coating System	
	Failure	10.4.34
10.4.52	Corroded Floorbeam End and Connection with	
	Deicing Chemical Residue	10.4.35
10.4.53	Corroded Stringers under an Open Grid Deck	10.4.36
10.4.54	Corroded End of Stringer	10.4.36
10.4.55	Corroded Floorbeams and Stringers	10.4.37
10.4.56	Collision Damage to Portal	10.4.38
10.4.57	Lateral Bracing with Corrosion	10.4.39
10.4.58	Sway Bracing with Pack Rust	10.4.38
10.4.59	Other Elements	10.4.40

Topic 10.5

Steel Arches

10.5.1	Deck Arch Bridge	10.5.1
10.5.2	Through Arch Bridge	10.5.1
10.5.3	Tied Arch Bridge	10.5.2
10.5.4	Deck Arch	10.5.3
10.5.5	Solid Ribbed Deck Arch	10.5.3
10.5.6	Braced Rib Deck Arch, New River Gorge, WV	10.5.4
10.5.7	Spandrel Braced Deck Arch with Six Arch Ribs	10.5.4
10.5.8	Hinge Pin at Skewback for Spandrel Braced Deck	10.5.5
10.5.9	Solid Ribbed Deck Arch Primary Members	10.5.5

Figure Nos.		Page Nos.
10.5.10	Solid Ribbed Deck Arch Secondary Members	10.5.6
10.5.11	Elevation View of a Braced Ribbed Through Arch	10.5.7
10.5.12	Through Arch Primary Members	10.5.8
10.5.13	Through Arch Secondary Members	10.5.8
10.5.14	Three-Span Tied Arch Bridge	10.5.9
10.5.15	Tied Arch Primary Members	10.5.10
10.5.16	Tied Arch Secondary Members	10.5.10
10.5.17	Tied Arch Bridge with Fracture Critical Eyebar Tie	
	Members	10.5.11
10.5.18	Floor System on a Through Arch	10.5.14
10.5.19	Through Truss Arch Members	10.5.14
10.5.20	Braced Rib Deck Arch Showing Spandrel Columns	10.5.15
10.5.21	Hanger Connection on a Through Arch	10.5.15
10.5.22	Performing Baseline Hardness Test on Fire	
	Damaged Arch Cables	10.5.16
10.5.23	Bracing Members in Deck Arch Bridge	10.5.19
10.5.24	Through Arch Member Exposed to Traffic	10.5.20

Topic 10.6 Steel Rigid Frames

10.6.1	Typical Rigid K-Frame Constructed of Two Frames.	10.6.1
10.0.2	Frames	10.6.2
10.6.3	Connection between Legs and Girder Portion	10.6.3
10.6.4	Delta Frame	10.6.4
10.6.5	Bearings	10.6.5
10.6.6	Transverse, Longitudinal, and Radial Stiffeners on a	10.6.5
	Frame Knee	
10.6.7	Two Frame Bridge with Floorbeam-Stringer Floor	
	System	10.6.6
10.6.8	Lateral Bracing for Frame Legs	10.6.7
10.6.9	Lateral Bracing and Diaphragms	10.6.8
10.6.10	Stress Zones in a Frame	10.6.8
10.6.11	Fracture Critical Structure - No Load Path	
	Redundancy	10.6.9
10.6.12	Multiple Frame Rigid Frame – Not a Fracture	
	Critical Structure	10.6.9
10.6.13	Bearing Area of a Two Frame Bridge	10.6.12
10.6.14	Flexural Zones (Greatest Bending Moment)	10.6.12

Page
Nos.

Topic 10.7 Pin and Hanger Assemblies

Figure Nos.

10.7.1	Typical Pin-and-Hanger Assembly	10.7.1
10.7.2	Single Pin with Riveted Pin Plate	10.7.2
10.7.3	Pin-and-Hanger Assembly Locations Relative to	
	Piers	10.7.2
10.7.4	Pin-and-Hanger Assembly	10.7.3
10.7.5	Pin Cap with Through Bolt	10.7.4
10.7.6	Threaded Pin with Retaining Nut	10.7.5
10.7.7	Plate Hanger and Eyebar Shape Hanger Link	10.7.5
10.7.8	Pin Cap, Through Bolt and Nut	10.7.6
10.7.9	Retaining Nut	10.7.6
10.7.10	Web Doubler Plates	10.7.7
10.7.11	Design Stress in a Hanger Link (Tension Only)	10.7.8
10.7.12	Actual Stress in a Hanger Link (Tension and	
	Bending)	10.7.8
10.7.13	Design Stress in a Pin (Shear and Bearing)	10.7.9
10.7.14	Actual Stress in a Pin (Shear, Bearing and Torsion)	10.7.9
10.7.15	Mianus River Bridge Failure	10.7.10
10.7.16	Multi-girder Bridge with Pin-and-Hanger	
	Assemblies	10.7.11
10.7.17	Ultrasonic Testing of a Pin	10.7.13
10.7.18	Alternate Hanger Link Retaining System	10.7.14
10.7.19	Pin Measurement Locations	10.7.15
10.7.20	Rust Stains from Pin Corrosion	10.7.16
10.7.21	Corroded Hanger Plate	10.7.17
10.7.22	Bowing Due to Out-of-Plane Distortion of Hanger	10.7.18
10.7.23	Fatigue Cracks in Pin-and-Hanger Assemblies	10.7.18
10.7.24	Corroded Pin and Hanger Assembly	10.7.19
10.7.25	Underslung Catcher Retrofit	10.7.21
10.7.26	Stainless Steel Pin-and-Hanger Assembly	10.7.21

Topic 10.8 Gusset Plates

10.8.1	Steel Truss Superstructure with Gusset Plates	10.8.1
10.8.2	Steel Deck Arch Superstructure with Stiffening	
	Truss and Gusset Plates	10.8.2
10.8.3	Steel Gusset Plate with Riveted Connections	10.8.3
10.8.4	Steel Gusset Plate with Welded Connections	10.8.3
10.8.5	Steel Gusset Plate with Riveted, Bolted and Welded	
	Connections	10.8.3

Figure		Page
Nos.		Nos.
10.8.6	Steel Gusset Plates Connecting Timber Primary	
	Truss Members	10.8.4
10.8.7	Odd-Shaped Gusset Plate Connecting Primary Load-	
	Carrying Truss Members	10.8.5
10.8.8	Gusset Plate Connecting Primary Load-Carrying	
	Truss Members	10.8.5
10.8.9	Potential Block Shear Rupture Planes for Gusset	
	Plates in Tension	10.8.7
10.8.10	Examples of Gross Section Shear Yielding Planes	10.8.8
10.8.11	Examples of Net Section Shear Fracture Planes	10.8.8
10.8.12	Example Showing the Unbraced Length and	
	Effective Width for a Gusset Plate in Compression	10.8.9
10.8.13	Examples of Combined Flexural and Axial Load	
	Planes	10.8.10
10.8.14	Gusset Plate Connecting Secondary (Bracing)	
	Members to a Primary Load-Carrying Truss	
	Member	10.8.11
10.8.15	Gusset Plate Connecting Secondary (Bracing)	
	Members on a Steel Two-Girder Bridge	10.8.11
10.8.16	Gusset Plate Field Measurements	10.8.14
10.8.17	General Corrosion of Gusset Plates	10.8.15
10.8.18	Corrosion Line Viewed from Inside and Outside of	
10010	Gusset Plate	10.8.16
10.8.19	Inspector Using a D-meter to Measure the Thickness	
100000	of the Gusset Plate	10.8.17
10.8.20	Inspector Using Calipers Measure the Thickness of	10010
10001	the Gusset Plate	10.8.18
10.8.21	Inspector Using a Straightedge and Tape to Measure	10010
10.0.00	the Section Loss of the Gusset Plate	10.8.18
10.8.22	V-WAC Gage and Inspector Using the V-WAC in	
	the Field to Measure the Section Loss of the Gusset	10010
10.0.00		10.8.19
10.8.23	Inspector Using a Portable Ultrasonic Testing	10 0 20
10.0.04	Inspection System	10.8.20
10.8.24	Ultrasonic Testing Inspection Acquisition Software	10.8.20
10.8.25	Cracked Gusset Plate and Point of Crack Initiation	10.8.21
10.8.26	Partial Length Cracked Tack Weld	10.8.22
10.8.27	Gusset Plate Buckling (Compression) Failure due to	10 0 00
10.0.00	Major Gusset Plate Section Loss	10.8.23
10.8.28	Gussel Plate with Paint Failure	10.8.24
10.8.29	Missing Boits on Gusset Plate	10.8.25
10.8.30	Flate Inickening and Free Edge Stiffening on	10 0 27
10 0 21	Uussel Male Waldad Datus ft	10.8.2/
10.8.51	roony Designed welded Ketrolit	10.8.2/

Figure		Page
Nos.		Nos.
10.8.32	Unbraced Gusset Plate Edges and Reference Line	10.8.28
10.8.33	Inspector Measuring Out-of-Plane Distortion Using	
	String Line and Tape Measure	10.8.29
10.8.34	Collapsed I-35W Mississippi River Bridge	10.8.32

Topic 10.9 Steel Eyebars

10.9.1	Typical Eyebar Tension Member on an Arch	10.9.1
10.9.2	Eyebar Cantilevered Truss Bridge (Queensboro	
	Bridge, NYC)	10.9.1
10.9.3	Eyebar Chain Suspension Bridge	10.9.2
10.9.4	Anchorage Eyebar	10.9.2
10.9.5	Collapsed Silver Bridge	10.9.3
10.9.6	Inspection of Eyebars	10.9.4
10.9.7	Retrofit of Eyebars to Add Redundancy	10.9.4
10.9.8	Eyebar Connection with Corrosion	10.9.5
10.9.9	Eads Bridge, St. Louis	10.9.6
10.9.10	Forged Loop Rod	10.9.7
10.9.11	Close-up of the End of a Loop Rod	10.9.7
10.9.12	Forged Eyebar by Mechanical forge Press	10.9.8
10.9.13	Eyebar Pin Hole (Disassembled Connection)	10.9.8
10.9.14	Eyebar Dimensions	10.9.9
10.9.15	Loosely Packed Eyebar Connection	10.9.10
10.9.16	Tightly Packed Eyebar Connection	10.9.10
10.9.17	Steel Pin Spacer or Filling Ring	10.9.11
10.9.18	Non-redundant Eyebar Member	10.9.12
10.9.19	Close-up of the Forge Zone on an Eyebar (Arrow	
	denotes crack)	10.9.15
10.9.20	Forge Loop is completely apart	10.9.15
10.9.21	Bowed Eyebar Member	10.9.16
10.9.22	Buckled Eyebar due to Abutment Movement	10.9.17
10.9.23	Corroded Spacer	10.9.18
10.9.24	Asymmetry at an Eyebar Connection	10.9.18
10.9.25	Eyebar Member with Unequal Load Distribution	10.9.19
10.9.26	Welds on Loop Rods	10.9.20
10.9.27	Welded Repair to Loop Rods	10.9.20
10.9.28	Turnbuckle on a Truss Diagonal	10.9.21
10.9.29	Welded Repair to Turnbuckles	10.9.21
10.9.30	Ultrasonic Inspection of Eyebar Pin	10.9.22
10.9.31	Fracture Critical Bottom Chord Truss Member:	
	Internally Non-redundant Eyebar	10.9.23
10.9.32	Fracture Critical Top Chord Truss Member:	
	Internally Redundant Eyebar	10.9.23
	-	

Topic 11.1 Bridge Bearings

11.1.1	Three Functions of a Bearing	11.1.1
11.1.2	Fixed and Movable Expansion Bearings	11.1.2
11.1.3	Elements of a Typical Bridge Bearing	11.1.3
11.1.4	Lubricated Steel Plate Bearing	11.1.5
11.1.5	Bronze Sliding Plate Bearing	11.1.6
11.1.6	Self-Lubricating Bronze Sliding Plate Bearing	11.1.7
11.1.7	Single Roller Bearing	11.1.8
11.1.8	Roller Nest Bearing	11.1.9
11.1.9	Rocker Bearing	11.1.8
11.1.10	Segmental Rocker Bearing	11.1.10
11.1.11	Segmental Rocker Nest Bearing	11.1.11
11.1.12	Pinned Rocker Bearing	11.1.12
11.1.13	Plain Neoprene Bearing Pad	11.1.13
11.1.14	Laminated Neoprene Bearing Pad	11.1.14
11.1.15	Neoprene Pot Bearing with Guide Bars	11.1.15
11.1.16	Disc Bearing	11.1.16
11.1.17	Fixed Bearing	11.1.16
11.1.18	Enclosed or Concealed Bearing	11.1.17
11.1.19	Pin and Link Bearing	11.1.18
11.1.20	Restraining Bearing	11.1.19
11.1.20	Restraining Bearing	11.1.19
11.1.21	Sketch of a Lead Core Isolation Bearing	11.1.22
11.1.22	Lead Core Isolation Bearing	11.1.20
11.1.23	Friction Pendulum Bearing	11.1.21
11.1.24	Sketch of a Friction Pendulum Bearing	11.1.22
11.1.25	Spherical Pot Bearing	11.1.23
11.1.26	Spalling of Concrete Bridge Seat Due to High Edge	
	Stress	11.1.25
11.1.27	Ultrasonic Testing Inspection of a Pin in a Bearing	11.1.26
11.1.28	Bent Anchor Bolt due to Excessive Horizontal	
	Movement	11.1.24
11.1.26	Uplift at Bridge Bearing	11.1.25
11.1.27	Sliding Plate Bearing Inspection Checklist Items	11.1.26
11.1.28	Heavy Corrosion on a Steel Rocker Bearing	11.1.27
11.1.29	Rocker Bearing with Excessive Horizontal	
	Movement	11.1.28
11.1.30	Bent Anchor Bolt due to Excessive Horizontal	
	Movement	11.1.29
11.1.31	Uplift at Bridge Bearing	11.1.29
11.1.32	Longitudinal Misalignment in Bronze Sliding Plate	
	Bearing	11.1.30

Figure		Page
Nos.		Nos.
11.1.33	Sliding Plate Bearing Inspection Checklist Items	11.1.31
11.1.34	Damaged Roller Nest Bearing	11.1.32
11.1.35	Rocker Bearing Inspection Checklist Items	11.1.33
11.1.36	Excessive Tilt in a Segmental Rocker	11.1.34
11.1.37	Frozen Rocker Nest	11.1.35
11.1.38	Frozen Rocker Nest	11.1.35
11.1.39	Elastomeric Bearing Inspection Checklist Items	11.1.37
11.1.40	Neoprene Bearing Pad Excessive Bulging	11.1.38
11.1.41	Lead Core Isolation Bearing	11.1.39
11.1.42	Serious Bearing Condition	11.1.41

Topic 12.1 Abutments and Wingwalls

12.1.1	Schematic of Common Abutment Types	12.1.3
12.1.2	Section View of Less Common Abutment Types	
	(Mechanically Stabilized Earth	12.1.4
12.1.3	Section View of Less Common Abutment Types	
	(Geosynthetic Reinforced Soil)	12.1.5
12.1.4	Full Height Abutment	12.1.6
12.1.5	Stub Abutment	12.1.6
12.1.6	Open Abutment	12.1.7
12.1.7	Integral Abutment	12.1.8
12.1.9	Mechanically Stabilized Earth Abutment (Note	12.1.10
	Precast Concrete Panels)	
12.1.10	Mechanically Stabilized Earth Wall Under	
	Construction	12.1.10
12.1.11	GRS Bridge Abutment at the FHWA Turner-	
	Fairbank Highway Research Center	12.1.11
12.1.12	View of the Founders/Meadows Bridge Supported	
	by GRS Abutments	12.1.12
12.1.13	Plain Unreinforced Concrete Gravity Abutment	12.1.12
12.1.14	Reinforced Concrete Cantilever Abutment	12.1.13
12.1.15	Stone Masonry Gravity Abutment	12.1.13
12.1.14	Combination: Timber Pile Bent Abutment with	
	Reinforced Concrete Cap	12.1.12
12.1.17	Steel Abutment	12.1.14
12.1.18	Primary Reinforcement in Concrete Abutments	12.1.15
12.1.19	Secondary Reinforcement in Concrete Abutments	12.1.15
12.1.20	Cheek Wall	10.1.16
12.1.21	Spread Footing/Deep Foundations	12.1.17
12.1.22	Stub Abutment on Piles with Piles Exposed	12.1.18
12.1.23	Typical Wingwall	12.1.19
12.1.24	Straight Wingwall	12.1.19
	6 6	

Figure		Page
Nos.		Nos.
12.1.25	Flared Wingwall	12.1.20
12.1.26	U-Wingwall	12.1.20
12.1.27	Integral Wingwall	12.1.21
12.1.28	Independent MSE Wingwall	12.1.21
12.1.29	Masonry Wingwall	12.1.22
12.1.30	Primary Reinforcement in Concrete Cantilever	
	Wingwall	12.1.22
12.1.31	Cracking in Bearing Seat of Concrete and Stone	
	Abutment	12.1.24
12.1.32	Spalled Concrete Wingwall	12.1.25
12.1.33	Cracking and Efflorescence in Abutment Backwall	12.1.25
12.1.34	Stone Masonry Abutment with Deteriorated Joints	12.1.26
12.1.35	Steel Abutment	12.1.26
12.1.36	Decay caused by insects in Timber Abutment	12.1.27
12.1.37	Local Failure in Timber Pile due to Lateral	
	Movement of Abutment	12.1.28
12.1.38	Decayed Lagging and Abrasion Caused by Scour of	
	a Timber Pile Bent Abutment	12.1.28
12.1.39	Differential Settlement between Different	
	Substructure Units	12.1.33
12.1.40	Differential Settlement Under an Abutment	12.1.33
12.1.41	Crack in Abutment due to Settlement	12.1.34
12.1.42	Lateral Movement of an Abutment due to Slope	
	Failure	12.1.34
12.1.43	Excessive Rocker Bearing Displacement Indicating	
	Possible Lateral Displacement of Abutment	12.1.35
12.1.44	Vertical Misalignment between Approach Slab (left)	
	and Bridge Deck (right)	12.1.36
12.1.45	Erosion at Abutment Exposing Footing	12.1.36
12.1.46	Rotational Movement of an Abutment	12.1.37
12.1.47	Rotational Movement at Abutment	12.1.38
12.1.48	Rotational Movement due to "Lateral Squeeze" of	
	Embankment Material	12.1.38
12.1.49	Rotational Movement at Concrete Wingwall	12.1.39
12.1.50	Abutment with Undermining due to Scour	12.1.41
12.1.51	Inspector Checking for Scour	12.1.41
12.1.52	Scour and Possible Undermining of Concrete	
	Wingwall	12.1.42

Topic 12.2 Piers and Bents

12.2.1	Example of Piers as Intermediate Supports for a	
	Bridge	12.2.1
12.2.2	Solid Shaft Pier	12.2.2

Figure		Page
Nos.		Nos.
12.2.3	Column Pier	12.2.2
12.2.4	Column Pier with Web Wall	12.2.3
12.2.5	Column Pier with Web Wall	12.2.3
12.2.6	Single Stem Pier (Cantilever or Hammerhead)	12.2.4
12.2.7	Cantilever Pier	12.2.4
12.2.8	Column Bent or Open Bent	12.2.5
12.2.9	Concrete Pile Bent.	12.2.5
12.2.10	Concrete Pier with Integral Steel Pier Cap	12.2.6
12.2.11	Integral Concrete Pier and Pier Cap	12.2.7
12.2.12	Integral Concrete Pier and Pier Cap	12.2.7
12.2.13	Reinforced Concrete Piers under Construction	12.2.8
12.2.14	Stone Masonry Pier	12.2.8
12.2.15	Steel Bent	12.2.9
12.2.16	Timber Pile Bent	12.2.9
12.2.17	Combination: Reinforced Concrete Column with	
	Steel Pier Cap	12.2.10
12.2.18	Primary Reinforcement in Column Bent with	12.2.10
12.2.10	Web Wall	12 2 11
12219	Secondary Reinforcement in Column Bent with Web	12.2.11
12.2.19	Wall	12 2 11
12 2 20	Primary Reinforcement in Column Bents	12.2.11
12.2.20	Primary Reinforcement for a Cantilevered Pier	12.2.12
12.2.21	Cantilevered Piers Joined by a Web Wall	12.2.12
12.2.22	Pile Rent	12.2.13 12.2.13
12.2.23 12 2 24	Collision Wall	12.2.14
12.2.24 12 2 25	Collision Wall	12.2.15
12.2.23	Concrete Block Dolphin	12.2.15
12.2.20	Timber Dolphin	12.2.10
12.2.27	Pier Fender	12.2.10
12.2.20	Fender System	12.2.17
12.2.27 12.2.27	Concrete Spalling due to Contaminated Drainage	12.2.17
12.2.30	Crack in Concrete Bent Can	12.2.17
12.2.31 12.2.31	Concrete Spalling on Bent Can	12.2.20
12.2.32	Collision Damage to Concrete Pier Column	12.2.20
12.2.33 12.2.33	Deterioreted and Missing Stone at Masonry Pier	12.2.21
12.2.34	Deterioration of Stool Port Log	12.2.22
12.2.33	Corresion of Steel Bile Bent at Water Surface	12.2.23
12.2.30	Steel Column Dile Port with Contilever High	12.2.23
12.2.37	Steer Column File Bent with Cantilever - High	12224
12 2 20	Stress Areas for Moment, Snear and Bearing	12.2.24
12.2.38	(Electrice)	12225
12 2 20	/ riashing)	12.2.23
12.2.39	Timber Bent Columns in Water	12.2.25
12.2.40	Decay of Timber Bent Column at Ground	10000
	Line/Loose Connection	12.2.26

Figure Nos.		Page Nos.
12.2.41	Timber Pile Bent with Overstress-Partial	1 (05)
	"Brooming" Failure at First Pile	12.2.26
12.2.42	Timber Pile Damage due to Limnoria Marine Borers	12.2.27
12.2.43	Timber Bent Damage due to Shipworm Marine	
	Borers	12.2.27
12.2.44	Differential Settlement between Different	
	Substructure Units	12.2.31
12.2.45	Differential Settlement Under a Pier	12.2.31
12.2.46	Superstructure Evidence of Pier Settlement	12.2.32
12.2.47	Cracks in Bent Cap due to Lateral Movement of	
	Bent during Earthquake	10.2.33
12.2.48	Pier Movement and Superstructure Damage due to	
	Scour/Undermining	12.2.33
12.2.49	Tipping of Bent due to Scour/Undermining	12.2.34
12.2.50	Repaired Concrete Column Bent	12.2.36
12.2.51	Fracture Critical Steel Bent	12.2.37
12.2.52	Concrete Dolphins	12.2.38
12.2.53	Steel Fender	12.2.38
12.2.54	Timber Fender System with Deteriorated Piles	12.2.39

Topic 13.1 Waterway Elements

13.1.1	Failure Due to High Water Levels During	
	Hurricane: Aerial View	13.1.2
13.1.2	Failure Due to High Water Levels During	
	Hurricane: Close-Up View	13.1.2
13.1.3	Pier Foundation Failure	13.1.3
13.1.4	Typical Waterway Cross Section Showing Well	
	Defined Channel Depression	13.1.5
13.1.5	Plan View of Rivers	13.1.6
13.1.6	Meandering River	13.1.7
13.1.7	Typical Floodplain	13.1.8
13.1.8	Hydraulic Waterway Opening	13.1.8
13.1.9	Crushed Stone Riprap	13.1.10
13.1.10	Spurs	13.1.11
13.1.11	Guide Banks Constructed on Kickapoo Creek Near	
	Peoria, Illinois	13.1.11
13.1.12	Gabion Basket Serving as Slope Protection	13.1.12
13.1.13	Slope Stabilization	13.1.12
13.1.14	Concrete Revetment Mat	13.1.13
13.1.15	Formed Concrete Channel Lining	13.1.13
13.1.16	Concrete Footing Apron on a Masonry Abutment	13.1.14

Figure		Page
Nos.		Nos.
13.1.17	Concrete Footing Apron to Protect a Spread Footing	
	from Undermining	13.1.14

Topic 13.2 Inspection of Waterways

1321	Flood Flow Around a Pier Showing High	
13.2.1	Streamflow Velocity	1322
13.2.2	Streambed Aggradation	13.2.4
13.2.3	Streambed Degradation	13.2.4
13.2.4	General Scour	13.2.5
12.2.5	Close-up of General Scour of a Pier	13.2.6
13.2.6	Stream Contraction Schematic	13.2.7
13.2.7	Contraction Scour Photograph	13.2.8
13.2.8	Large number of Piers Combine to Reduce the	
	Hvdraulic Opening	13.2.8
13.2.9	Vegetation Constricting the Waterway	13.2.9
13.2.10	Sediment Deposits Within the Waterway Opening	13.2.9
13.2.11	Ice in Stream Resulting in Possible Contraction	
	Scour	13.2.10
13.2.12	Debris Build-up in the Waterway	13.2.10
13.2.13	Local Scour at a Pier	13.2.12
13.2.14	Local Scour at a Pier	13.2.12
13.2.15	Wide Pier	13.2.13
13.2.16	Long Pier	13.2.13
13.2.17	Lateral Stream Migration endangering an Abutment.	13.2.15
13.2.18	Streambank Damage	13.2.16
13.2.19	Sloughing Streambank	13.2.16
13.2.20	Undermined Streambank	13.2.17
13.2.21	Stream Meander Changes	13.2.17
13.2.22	Channel Widening	13.2.18
13.2.23	Schematic of Noncohesive Bank Material	13.2.19
13.2.24	Schematic of Cohesive Bank Material	13.2.19
13.2.25	Schematic of Cohesive Bank Material	13.2.19
13.2.26	End and Side View of Scour and Undermining	13.2.21
13.2.27	Pier Settlement due to Undermining	13.2.22
13.2.28	Probing Rod and Waders	13.2.24
13.2.29	Surface Supplied Air Diving Equipment	13.2.25
13.2.30	Rapid Flow Velocity	13.2.26
13.2.31	Navigable Waterway	13.2.26
13.2.32	Streambed Cross-Section	13.2.27
13.2.33	Streambed Profile	13.2.28
13.2.34	Scour Monitoring Collar	13.2.29

Figure Nos.		Page Nos.
13.2.35	Pile Bent Deterioration Normally Hidden	13.2.30
	Underwater	
13.2.36	Out of Plumb Pier Column	13.2.30
13.2.37	Superstructure Misalignment	13.2.31
13.2.38	Drift Lodged in a Superstructure	13.2.32
13.2.39	Multi-Span Simply Supported Bridge	13.2.32
13.2.40	Failed Riprap	13.2.33
13.2.41	Severe Streambed Degradation Evident at Low	13.2.34
	Water	
13.2.42	Approach Roadway Built in the Floodplain	13.2.35
13.2.43	Stable Banks	13.2.35
13.2.44	Sediment Accumulation Redirecting Streamflow	13.2.36
13.2.45	Fence in Stream at Bridge	13.2.37
13.2.46	Waterway Alignment 1990 – 2006	13.2.38
13.2.47	Approach Spans in the Floodplain	13.2.39
13.2.48	Debris and Sediment in the Channel	13.2.40
13.2.49	Upstream Dam	13.2.41
13.2.50	Scour at a Pile Abutment	13.2.42
13.2.51	Fast Flowing Stream	13.2.43
13.2.52	Scour Rates vs. Velocity for Common Streambed	
	Materials	13.2.43
13.2.53	Typical Misaligned Waterway	13.2.44
13.2.54	Typical Large Floodplain	13.2.44
13.2.55	Lateral Stream Migration	13.2.45
13.2.56	Stream Alignment Not Parallel with Abutments	13.2.46
13.2.57	Rotational Movement and Failure Due to	
	Undermining	13.2.46
13.2.58	Exposed Piling Due to Scour	13.2.47
13.2.59	Accelerated Flow Due to Constricted Waterway	13.2.47
13.2.60	Scour Assessment – Safe	13.2.49
13.2.61	Scour Assessment – Evaluate	13.2.50
13.2.61	Scour Assessment – Fix	13.2.50
13.2.63	(Exhibit 63) Culvert Failure Due to Overtopping	13.2.54
13.2.64	(Exhibit 64) Culvert Almost Completely Blocked by	
	Sediment Accumulation	13.2.54
13.2.65	(Exhibit 65) Drift and Debris Inside Timber Box	
	Culvert	13.2.55

Topic 13.3 Underwater Inspection

13.3.1	Schoharie Creek Bridge Failure	13.3.1
13.3.2	Liberty Bridge over Monongahela River	13.3.2
13.3.3	Level II Cleaning of a Steel Pile	13.3.5

Figure		Page
Nos.		Nos.
13.3.4	Diver Cleaning Pier Face For Inspection	13.3.5
13.3.5	Channel Cross-Section (Current Inspection Versus	
	Original Channel)	13.3.7
13.3.6	Pier Sounding Grid	13.3.7
13.3.7	Permanent Reference Point (Bolt Anchored in Nose	
	of the Pier, Painted Orange)	13.3.8
13.3.8	Local Scour; Causing Undermining of a Pier	13.3.8
13.3.9	Bascule Bridge on the Saint Croix River	13.3.9
13.3.10	Flood Conditions: Pier Settlement	13.3.11
13.3.11	Buildup of Debris At Pier	13.3.11
13.3.12	Movement of a Substructure Unit	13.3.12
13.3.13	Bridge Owner's Underwater Inspection Plan	
	Checklist	13.3.17
	Bridge Owner's Underwater Inspection Plan	
	Checklist (cont'd.)	13.3.18
	Bridge Owner's Underwater Inspection Plan	
	Checklist (cont'd.)	13.3.19
	Bridge Owner's Underwater Inspection Plan	
	Checklist (cont'd.)	13.3.20
13.3.14	Timber Pile Bent	13.3.21
13.3.15	Steel Pile Bent	13.3.22
13.3.16	Concrete Pile Bent	13.3.22
13.3.17	Column Pier with Solid Web Wall	13.3.23
13.3.18	Cantilever or Hammerhead Pier	13.3.23
13.3.19	Solid Shaft Pier	13.3.24
13.3.20	Severe Flood-Induced Abutment Scour	13.3.25
13.3.21	Damaged Protective System	13.3.26
13.3.22	Inspection of Culvert With Limited Freeboard and	
	Ice Cover	13.3.27
13.3.23	Concrete Deterioration	13.3.28
13.3.24	Deteriorated Timber Piling	13.3.29
13.3.25	Deteriorated Steel Piles at Splash Zone	13.3.30
13.3.26	Sample Underwater Inspection Form	13.3.33
13.3.26	Sample Underwater Inspection Form (Continued)	13.3.34
13.3.27	Diving Inside a Cofferdam	13.3.35
13.3.28	Excessive Current	13.3.36
13.3.29	Debris	13.3.36
13.3.30	Cleaning a Timber Pile	13.3.37
13.3.31	Commercial Marine Traffic	13.3.38
13.3.32	Alpha (top) and Sport Diver (bottom) Flags	13.3.39
13.3.33	Inspector Performing a Wading Inspection	13.3.40
13.3.34	SCUBA Inspection Diver	13.3.40
13.3.35	Surface-Supplied Diving Inspection	13.3.41
13.3.36	Vulcanized Rubber Dry Suit	13.3.43

Figure Nos		Page Nos
13.3.37	Full Face Lightweight Diving Mask with	1105.
10.0.07	Communication System	13.3.43
13.3.38	Surface-Supplied Air Equipment, Including Air	10.01.10
10.0.00	Compressor, Volume Tank With Air Filters, and	
	Umbilical Hoses	13.3.44
13.3.39	Surface-Supplied Diving Equipment Including	10.0111
	Helmet or Hard Hat	13.3.44
13.3.40	Pneumofathometer Gauge	13.3.45
13.3.41	Surface-Supplied Diver with a Reserve Air Tank	13.3.45
13.3.42	Wireless Communication Box System	13.3.46
13.3.43	Surface Communication With Inspection Team	
	Leader	13.3.47
13.3.44	Access Barge and Exit Ladder	13.3.47
13.3.45	Access From Dive Boat	13.3.48
13.3.46	Diver with a Pry Bar and Diver with Hand Scraper	13.3.49
13.3.47	Cleaning with a Water Blaster	13.3.50
13.3.48	Coring Equipment	13.3.51
13.3.49	Concrete Coring Taking Place	13.3.52
13.3.50	Concrete Core	13.3.52
13.3.51	Timber Core	13.3.53
13.3.52	Various Waterproof Camera Housings	13.3.54
13.3.53	Diver Using a Camera in a Waterproof Housing	13.3.54
13.3.54	Diver Using a Clearwater Box	13.3.55
13.3.55	Underwater Video Inspection	13.3.56
13.3.56	Remotely Operated Vehicle (ROV)	13.3.56
13.3.57	Acoustic Imaging of a Pier	13.3.57
13.3.58	Ground Penetrating Radar Record	13.3.59
13.3.59	Tuned Transducer Record	13.3.60
13.3.60	Pier Undermining, Exposing Timber Foundation	
	Pile	13.3.61

Topic 14.1 Culvert

Characteristics

14.1.1	Culvert Structure	14.1.1
14.1.2	Box Culvert with Shallow Cover	14.1.5
14.1.3	AASHTO Wheel Loads and Wheel Spacings	14.1.7
14.1.4	AASHTO Wheel Load Surface Contact Area (Foot	
	Print)	14.1.8
14.1.5	Spread Load Area (Single Dual Wheel)	14.1.8
14.1.6	Culvert Construction and Installation Requirements	14.1.10
14.1.7	Circular Culvert Structure	14.1.11
14.1.8	Pipe Arch Culvert	14.1.12
14.1.9	Arch Culvert	14.1.12

Figure		Page
Nos.		Nos.
14.1.10	Concrete Box Culvert	14.1.13
14.1.11	Multiple Cell Concrete Culvert	14.1.14
14.1.12	Frame Culvert	14.1.14
14.1.13	Large Structural Plate Pipe Arch Culvert	14.1.16
14.1.14	Large Structural Plate Box Culvert	14.1.16
14.1.15	Stone Masonry Arch Culvert	14.1.17
14.1.16	Timber Box Culvert	14.1.17
14.1.17	Schematic of a Single Walled Plastic Culvert	14.1.18
14.1.18	Culvert End Projection	14.1.19
14.1.19	Culvert Mitered End	14.1.19
14.1.20	Culvert Skewed End	14.1.20
14.1.21	Culvert Headwall and Wingwalls	14.1.20
14.1.22	Apron	14.1.21
14.1.23	Riprap Basin	14.1.22
14.1.24	Factors Affecting Culvert Discharge (Source:	
	Concrete Pipe Design Manual, American Concrete	
	Pipe Association, April 2007	14.1.24
14.1.25	Bending or Shear Failure	14.1.26
14.1.26	Cracking of Culvert End Treatment Due to	
	Foundation Settlement	14.1.27
14.1.27	Scour and Undermining at Culvert Inlet	14.1.27
14.1.28	Debris and Sediment Buildup	14.1.28
14.1.29	Approach Roadway at a Culvert Site	14.1.29
14.1.30	Repaired Roadway Over a Culvert	14.1.30
14.1.31	Slide Failure	14.1.31
14.1.32	Headwall and Wingwall End Treatment on Box	
	Culvert	14.1.32
14.1.33	Potential for Tilted Wingwalls	14.1.32
14.1.34	Skewed End	14.1.33
14.1.35	Culvert Headwall and Wingwall End Treatment	14.1.35
14.1.36	Apron	14.1.34
14.1.37	Energy Dissipater	14.1.34

Topic 14.2 Rigid Culverts

1421	Rigid Culvert	1421
14.2.2	Concrete Box Culvert	14.2.2
14.2.3	Multi-Cell Concrete Box Culvert	14.2.2
14.2.4	Precast Concrete Box Culvert	14.2.3
14.2.5	Concrete Pipe Culvert	14.2.4
14.2.6	Twin Concrete Pipe Culvert	14.2.4
14.2.7	Concrete Arch Culvert	14.2.5
14.2.8	Concrete Frame Culvert	14.2.6
14.2.9	Stone Masonry Arch Culvert	14.2.6

Figure Nos		Page Nos
14 2 10	Timber Box Culvert	14 2 7
14 2 11	Loads on a Concrete and Timber Box Culvert	14 2 8
14.2.12	Steel Reinforcement in a Concrete Box Culvert	14.2.10
14.2.13	Precast Box Section with Post-tensioning Steel	14.2.10
1	Ducts	1
14.2.14	Steel Reinforcement in a Concrete Arch Culvert	14.2.10
14.2.15	Steel Reinforcement in a Concrete Pipe Culvert	14.2.11
14.2.16	Sighting Along Culvert Sidewall to Check	
	Horizontal Alignment	14.2.17
14.2.17	Spalls and Delaminations on Top Slab of Concrete	
	Box Culvert	14.2.18
14.2.18	Missing Stones in Masonry Culvert	14.2.18
14.2.19	Precast Concrete Box Culvert Joint	14.2.20
14.2.20	Longitudinal Cracks in Pipe Culvert	14.2.20
14.2.21	Transverse Cracks in Pipe Culvert	14.2.21
14.2.22	Shear Slabbing (Source: FHWA Culvert Inspection	
	Manual)	14.2.22
14.2.23	Cast-in-Place Concrete Headwall and Wingwall	14.2.23
14.2.25	Standard Sizes for Concrete Pipe (Source: American	
а	Concrete Pipe Association)	14.2.27
14.2.25	Standard Sizes for Concrete Pipe (Source: American	
b	Concrete Pipe Association)	14.2.28
14.2.25	Standard Sizes for Concrete Pipe (Source: American	
с	Concrete Pipe Association)	14.2.29
14.2.26	Standard Concrete Pipe Shapes (Source: FHWA	
	Culvert Inspection Manual, Supplement to the	
	BIRM, July 1986)	14.2.31

Topic 14.3 Flexible Culverts

]	14.3.1	Pipe Arch Flexible Culvert	14.3.1
]	14.3.2	Flexible Box Culvert	14.3.2
1	14.3.3	Flexible Culvert: Load vs. Shape	14.3.2
]	14.3.4	Formula for Ring Compression	14.3.3
]	14.3.5	(Exhibit 11 Culvert Inspection Manual Report No.	
		FHWA-IP-86-2) Standard Corrugated Steel Culvert	
		Shapes (Source: Handbook of Steel Drainage and	
		Highway Construction Products, American Iron and	
		Steel Institute)	14.3.4
]	14.3.6	Schematic of a Single Walled Culvert	14.3.6
]	14.3.7	Schematics of Dual Walled Culverts	14.3.6
]	14.3.8	(Exhibit 66) Checking Curvature by Curve and	
		Middle Ordinate	14.3.14

Figure Nos.		Page Nos.
14.3.9	(Exhibit 67) Surface Indications of Infiltration	14.3.15
14.3.10	(Exhibit 68) Surface Hole Above Open Joint	14.3.16
14.3.11	(Exhibit 69) Close-Up of Loose and Missing Bolts at	
	a Cusped Seam; Loose Fasteners are Usually	
	Detected by Tapping the Nuts with a Hammer	14.3.17
14.3.12	(Exhibit 70) Cocked Seam with Cusp Effect	14.3.18
14.3.13	(Exhibit 71) Cracking Due to Deflection	14.3.19
14.3.14	(Exhibit 72) Circumferential Seam Failure Due to	
1 4 9 1 5	Embankment Slippage	14.3.20
14.3.15	(Exhibit 73) Suggested Rating Criteria for Condition	14222
14216	OF Corrugated Metal	14.3.22
14.3.10	(Exhibit 74) Perforation of the invert Due to	1/ 3 22
14317	(Exhibit 75) Invert Deterioration	14 3 23
14.3.17	(Exhibit 76) Differential Footing Settlement	14.3.23
14.3.10	(Exhibit 77) Footing Rotation Due to Undermining	14 3 24
14.3.20	(Exhibit 78) Erosion of Invert Undermining footing	11.3.21
1	of Arch	14.3.25
14.3.21	(Exhibit 79) Erosion Damage to Concrete Invert	14.3.26
14.3.22	(Exhibit 80) Excessive Side Deflection	14.3.28
14.3.23	(Exhibit 81) Shape Inspection Circular and Vertical	
	Elongated Pipe	14.3.29
14.3.24	(Exhibit 82) Condition Rating Guidelines	14.3.31
14.3.25	(Exhibit 83) Bottom Distortion in Pipe Arches	14.3.32
14.3.26	(Exhibit 84) Bottom and Corners of this Pipe Arch	
	have Settled	14.3.33
14.3.27	(Exhibit 85) Shape Inspection Structural Plate Pipe	
	Arch.	14.3.34
14.3.28	(Exhibit 86) Condition Rating Guidelines	14.3.35
14.3.29	(Exhibit 87) Arch Deflection During Installation	14.3.36
14.3.30	(Exhibit 88) Racked and Peaked Arch	14.3.3/
14.3.31	(Exhibit 89) Shape Inspection Structural Plate Arch	14.3.38
14.3.32	(Exhibit 90) Condition Rating Guidelines	14.3.39
14.3.33	(Exhibit 91) Shape inspection Structural Plate Box	14240
1/23/	(Exhibit 02) Condition Pating Guidelines	14.5.40
14.3.34	(Exhibit 92) Condition Rating Outdennes	14.3.42
14.3.35	(Exhibit 93) Typical Long-Span Shapes	14.3.43
14.3.30	(Exhibit 94) Elosion Danage to Concrete Invert (Exhibit 95) Shape Inspection Crown Section of	17.3.77
17.J.J/	Long Snan Structures	14345
14.3.38	(Exhibit 96) Shape Inspection Low Profile Long	11.3.43
	Span Arch	14.3.47
14.3.39	(Exhibit 97) Condition Rating Guidelines	14.3.48

Figure		Page
Nos.		Nos.
14.3.40	(Exhibit 98) Shape Inspection High Profile Long-	142.50
1 4 2 41	Span Arch	14.3.50
14.3.41	(Exhibit 99) Condition Rating Guidelines	14.3.51
14.3.42	(Exhibit 100) Shape Inspection Long-Span	1 4 2 52
1 4 9 49	Horizontal Ellipse	14.3.52
14.3.43	(Exhibit 101) Condition Rating Guidelines	14.3.53
14.3.44	(Exhibit 102) Potential for Differential Settlement in	
1 4 9 4 5	Horizontal Ellipse	14.3.54
14.3.45	(Exhibit 103) Shape Inspection Long-Span	1 4 9 5 5
	Horizontal Ellipse	14.3.55
14.3.46	(Exhibit 104) Condition Rating Guidelines	14.3.56
14.3.47	Standard Sizes for Corrugated Steel Culverts	
	(Source: American Iron and Steel Institute)	14.3.57
14.3.47	Standard Sizes for Corrugated Steel Culverts	
	(Source: American Iron and Steel Institute),	
	continued	14.3.58
14.3.47	Standard Sizes for Corrugated Steel Culverts	
	(Source: American Iron and Steel Institute),	
	continued	14.3.59
14.3.47	Standard Sizes for Corrugated Steel Culverts	
	(Source: American Iron and Steel Institute),	
	continued	14.3.59
14.3.47	Standard Sizes for Corrugated Steel Culverts	
	(Source: American Iron and Steel Institute),	
	continued	14.3.60
14.3.47	Standard Sizes for Corrugated Steel Culverts	
	(Source: American Iron and Steel Institute),	
	continued	14.3.61
14.3.47	Standard Sizes for Corrugated Steel Culverts	
	(Source: American Iron and Steel Institute),	
	continued	14.3.62
14.3.47	Standard Sizes for Corrugated Steel Culverts	
	(Source: American Iron and Steel Institute),	
	continued	14.3.63
14.3.47	Standard Sizes for Corrugated Steel Culverts	
	(Source: American Iron and Steel Institute),	
	continued	14.3.64
14.3.47	Standard Sizes for Corrugated Steel Culverts	
	(Source: American Iron and Steel Institute),	
	continued	14.3.65
14.3.47	Standard Sizes for Corrugated Steel Culverts	
	(Source: American Iron and Steel Institute),	
	continued	14.3.66

Figure		Page
Nos.		Nos.
14.3.47	Standard Sizes for Corrugated Steel Culverts	
	(Source: American Iron and Steel Institute),	
	continued	14.3.67
14.3.47	Standard Sizes for Corrugated Steel Culverts	
	(Source: American Iron and Steel Institute),	
	continued	14.3.68
14.3.47	Standard Sizes for Corrugated Steel Culverts	
	(Source: American Iron and Steel Institute),	
	continued	14.3.69
14.3.47	Standard Sizes for Corrugated Steel Culverts	
	(Source: American Iron and Steel Institute),	
	continued	14.3.70
14.3.47	Standard Sizes for Aluminum Culvert (Source:	
	American Iron and Steel Institute), continued	14.3.71
14.3.48	Standard Sizes for Aluminum Culvert (Source:	
	Aluminum Association), continued	14.3.72
14.3.48	Standard Sizes for Aluminum Culvert (Source:	
	Aluminum Association), continued	14.3.73
14.3.48	Standard Sizes for Aluminum Culvert (Source:	
	Aluminum Association), continued	14.3.74
14.3.48	Standard Sizes for Aluminum Culvert (Source:	
	Aluminum Association), continued	14.3.75
14.3.48	Standard Sizes for Aluminum Culvert (Source:	
	Aluminum Association), continued	14.3.76
14.3.48	Standard Sizes for Aluminum Culvert (Source:	
	Aluminum Association), continued	14.3.77
14.3.48	Standard Sizes for Aluminum Culvert (Source:	
	Aluminum Association), continued	14.3.78
14.3.48	Standard Sizes for Aluminum Culvert (Source:	
	Aluminum Association), continued	14.3.79
14.3.48	Standard Sizes for Aluminum Culvert (Source:	
	Aluminum Association), continued	14.3.80
14.3.48	Standard Sizes for Aluminum Culvert (Source:	
	Aluminum Association), continued	14.3.81
14.3.48	Standard Sizes for Aluminum Culvert (Source:	
	Aluminum Association), continued	14.3.82
14.3.48	Standard Sizes for Aluminum Culvert (Source:	
	Aluminum Association), continued	14.3.83
14.3.48	Standard Sizes for Aluminum Culvert (Source:	
	Aluminum Association), continued	14.3.84
14.3.48	Standard Sizes for Aluminum Culvert (Source:	
	Aluminum Association), continued	14.3.85
14.348	Standard Sizes for Aluminum Culvert (Source:	
	Aluminum Association), continued	14.3.86

Figure	Page
Nos.	Nos.

Topic 15.1 Timber

15.1.1	Sonic Testing Equipment	15.1.2
15.1.2	Stress Wave Timer	15.1.3
15.1.3	Ultrasonic Testing Equipment	15.1.4
15.1.4	Vibration Testing Equipment	15.1.5
15.1.5	Timber Boring Tool	15.1.6
15.1.6	Inspector Using Decay Detection Device	15.1.7
15.1.7	Moisture Content Equipment	15.1.8
15.1.8	Pick Test: Sound Wood, Decayed Wood	15.1.9
15.1.9	Field Ohmmeter Equipment	15.1.9

Topic 15.2 Concrete

15.2.1	Portable Hand Held Sonic/Ultrasonic Testing Sensor	
	Array System	15.2.2
15.2.2	Acoustic Emission Sensors	15.2.2
15.2.3	Half-Cell Potential	15.2.3
15.2.4	Delamination Detection Machinery	15.2.4
15.2.5	Schematic of Ground Penetrating Radar	15.2.5
15.2.6	The HERMES Bridge Inspector (Outside)	15.2.6
15.2.7	The HERMES Bridge Inspector (Inside)	15.2.7
15.2.8	Impact-Echo Testing Equipment	15.2.8
15.2.9	Deck with Area of Delamination (Warmer Colors)	15.2.9
15.2.10	Infrared Thermography Testing Equipment	15.2.9
15.2.11	Schematic of Thermal Imaging	15.2.10
15.2.12	Pachometer Testing Equipment	15.2.11
15.2.13	Remote Video Inspection Device	15.2.13

Topic 15.3 Steel

Acoustic Sensors Used to Determine Crack	
Propagation	15.3.2
Inspector Using Acoustic Emissions to Determine	
Crack Propagation	15.3.3
Detection of a Crack Using Dye Penetrant	15.3.4
Magnetic Particle Device Used to Detect Subsurface	
Flaws	15.3.5
Schematic of Magnetic Field Disturbance	15.3.6
Radiographic Testing	15.3.7
Robotic Inspection: Unmanned and Underwater	
Inspection Vehicles	15.3.8
Ultrasonic Testing of a Pin in a Moveable Bridge	15.3.9
	Acoustic Sensors Used to Determine Crack Propagation Inspector Using Acoustic Emissions to Determine Crack Propagation Detection of a Crack Using Dye Penetrant Magnetic Particle Device Used to Detect Subsurface Flaws Schematic of Magnetic Field Disturbance Radiographic Testing Robotic Inspection: Unmanned and Underwater Inspection Vehicles Ultrasonic Testing of a Pin in a Moveable Bridge

Figure Nos.		Page Nos.
15.3.9	Ultrasonic Thickness Depth Meter (D-meter)	15.3.10
15.3.10	Ultrasonic Testing of a Gusset Plate Using a	
	Portable UT	15.3.10
15.3.11	Hand Held Eddy Current Testing (ET) Instruments	15.3.11
15.3.12	Electrochemical Fatigue Sensor	15.3.12
15.3.13	Charpy V-Notch Test	15.3.13
15.3.14	Brittle Failure of Cast Iron Specimen	15.3.15
15.3.15	Ductile Failure of Cold Rolled Steel	15.3.15

Topic 15.4 Advanced Asset Assessment

15.4.1	Installation of Sensors	15.4.1
15.4.2	Viewing Real time Data	15.4.2
15.4.3	Strain Gage Used on the Hoan Bridge Milwaukee,	
	Wisconsin	15.4.3
15.4.4	Dynamic Load Testing Vehicle	15.4.5
15.4.5	Structural Model	15.4.6

Topic 16.1 Cable Supported Bridges

16.1.1	Golden Gate Bridge	16.1.1
16.1.2	Maysville Cable-Stay Bridge	16.1.2
16.1.3	Roebling Bridge	16.1.3
16.1.4	Sunshine Skyway Cable-Stayed Bridge in Tampa	
	Bay, Florida	16.1.3
16.1.5	Parallel Wire	16.1.5
16.1.6	Structural Wire Strand	16.1.5
16.1.7	Structural Wire Rope	16.1.5
16.1.8	Parallel Strand Cable	16.1.5
16.1.9	Locked Coil Strand Cross-Section	16.1.6
16.1.10	Parallel Wire	16.1.6
16.1.11	Parallel Strand	16.1.7
16.1.12	Cable Wrapping on the Wheeling Suspension	
	Bridge	16.1.7
16.1.13	Shapes of Towers Used for Cable Stay Bridges	16.1.8
16.1.14	Tower Types: Concrete "Portal Tower" and "A-	16.1.8
	Frame Tower"	
16.1.15	Tower Types: Steel "Portal Tower" and Concrete	
	"Single Column Tower"	16.1.9
16.1.16	Three-Span Suspension Bridge Schematic	16.1.9
16.1.17	Anchor Block Schematic	16.1.10

Figure		Page Nos
16 1 18	Cable Saddles for the Manhattan Bridge NYC	1105.
10.1.10	(Main span 1 480 ft)	16 1 11
16119	Grooved Cable Band	16 1 11
16.1.19	Open Socket Suspender Cable Connection	16 1 12
16.1.20	Cable Vibrations Local System Schematic	16.1.12
16 1 22	Cable Vibrations Global System Schematic	16 1 12
16.1.22	Cable Damping System - Wheeling Systemsion	10.1.12
10.1.25	Bridge – (Photo Courtesy of Geoffrey H. Goldberg	
	1999)	16 1 13
16 1 24	Cable Tie Damper System	16 1 13
16.1.21	Radial or Converging Cable System Schematic	16 1 14
16.1.25	Harn or Parallel Cable System Schematic	16 1 15
16.1.20	Fan or Intermediate Cable System Schematic	16 1 15
16.1.27	Star Cable System Schematic	16 1 16
16.1.20	Single Vertical Plane Cable System	16 1 16
16.1.2)	Double Vertical Plane Cable System	16 1 17
16.1.30	Double Inclined Plane Cable System	16 1 18
16.1.32	Cable Saddle	16 1 19
16 1 33	Cable Deck Anchorage	16 1 20
16 1 34	Anchor Inspection on Veteran's Bridge	16.1.20
16.1.35	Damper on Cable Staved Bridge	16.1.21
16.1.36	Anchor Block Schematic	16.1.23
16.1.37	Anchorage Interior of Ben Franklin Bridge.	1011120
101107	Philadelphia PA	16.1.24
16.1.38	Tape and Rubber Seal Torn Around Cable	1011.2
1011100	Allowing Water Penetration into Top of Sheath	16.1.25
16.1.39	Form for Recording Deficiencies in the Cable	1011120
	System of a Suspension Bridge	16.1.26
16.1.40	Cable-Staved Bridge	16.1.27
16.1.41	Cable-Staved Bridge Cables	16.1.28
16.1.42	Cable Wrapping Placement	16.1.29
16.1.43	Deformed Cable Wrapping	16.1.29
16.1.44	Corrosion of Steel Sheathing	16.1.30
16.1.45	Bulging of Cable Sheathing	16.1.30
16.1.46	Cracking of Cable Sheathing	16.1.31
16.1.47	Splitting of Cable Sheathing	16.1.31
16.1.48	Shock Absorber Damper System	16.1.32
16.1.49	Shock Absorber Damper System	16.1.32
16.1.50	Cable Tie Type Damper System	16.1.33
16.1.51	Tuned Mass Damper System	16.1.34
16.1.52	Neoprene Boot at Steel Anchor Pipe Near Anchor	16.1.35
16.1.53	Split Neoprene Boot	16.1.35
16.1.54	Corrosion of the Anchor System	16.1.36

	Figure Nos.		Page Nos.
	16.1.55	Form for Recording Deteriorations in Cable System of a Cable-Stayed Bridge	16.1.37
Topic 16.2 Movable Bridges			
Druges	1621	Movable Bridge	1621
	16.2.1	Typical "Permit Drawing" Showing Channel Width	10.2.1
	10.2.2	and Underclearance in Closed and Open Position	1622
	1623	The First All Iron Moyable Bridge in the Midwest	10.2.2
	10.2.5	was Completed in 1850 (Photo on File at the	
		Chicago Historical Society)	1622
	1624	Contago Instancia Society)	16.2.3
	16.2.4	Center Dearing Swing Druge	16.2.4
	16.2.5	Levent of Contor Dearing Type Swing Span with	10.2.3
	10.2.0	Mashing on the Span	1625
	1627	Pagoula Dridgo in the Open Desition	162.5
	16.2.7	Dascule Diluge III lie Open Position	16.2.0
	16.2.0	Double L of Polling Lift Posculo	16.2.7
	16.2.9	Trunnian Descule Pridge Schematic	16.2.0
	16.2.10	Double L of Trunnian Descule Dridge	16.2.9
	16.2.11	Each Trunnion is Supported on Two Decimes	16.2.9
	16.2.12	Each Trunnion is Supported on Two Bearings	10.2.10
	10.2.13	Sharm) is Leasted Outside of the Decoule Traces	
		Snown) is Located Outside of the Bascule Trusses	16 2 10
	16 2 14	Multi Transion, Strayge Tyme Decoule Dridge	16.2.10
	16.2.14	Wartigel Lift Dridge Schematic	16.2.11
	16.2.15	Vertical Lift Dridge Schematic	10.2.12
	10.2.10	the Lift Trace Spen and the Operating Drame Detete	
		the Wind the He Heyl (Lifting) Dense of They	
		Simultaneously Unwind the Devue Head Perce	16 2 12
	16 2 17	Simultaneously Unwind the Down-Haul Ropes	10.2.13
	10.2.17	Toward and the Dim Coord (and Operating Sheaver)	
		rowers, and the Rim Gears (and Operating Sneaves)	16 2 12
	16 2 19	Vertical Lift Dridge swith Devren and Drive Systems	10.2.13
	10.2.18	on Toward	16214
	16 2 10	On Towers	10.2.14
	16.2.19	Open Gearing	16.2.14
	16.2.20	Speed Reducer	16.2.13
	10.2.21	Coupling	10.2.13
	10.2.22	Dearing	10.2.10
	10.2.23	Silve Type Break	10.2.1/
	10.2.24	Spring Set Hydrauncany Keleased Disc Break	10.2.1/
	10.2.23	Low Speed High Torque Hydraulic Motor	10.2.18
	10.2.26	AC Emergency Motor	10.2.19

Figure		Page
Nos.		Nos.
16.2.27	Air Buffer	16.2.19
16.2.28	Shock Absorber	16.2.20
16.2.29	Typical Air Buffer Schematic	16.2.21
16.2.30	Typical Mechanically Operated Span Lock	16.2.22
16.2.31	Hydraulic Cylinder that Drives Lock Bars	16.2.22
16.2.32	Concrete Counterweight on a Single-Leaf Bascule	10.2.22
10.2.02	Bridge	16.2.23
16.2.33	Concrete Counterweight on a Vertical Lift Bridge	16.2.23
16.2.34	Closed Span Resting on Live Load Shoes	16.2.24
16.2.35	Traffic Barrier	16 2 24
16.2.35	Center Pivot Bearing	16.2.21
16.2.30	Balance Wheel in-place over Circular Rack	16.2.26
16.2.38	End Wedge	16 2 27
16 2 39	Hydraulic Cylinder Actuator	16.2.27
16.2.37	End Wedges Withdrawn and End Latch Lifted	16.2.27
16 2 41	Circular Lift Tread and Track Castings	16.2.20
16 2 42	Rack Casting and Pinion	16.2.2)
16.2.12	Rack Casting Ready for Fabrication	16 2 31
16 2 44	Drive Pinion	16 2 31
16 2 45	Trunnion Bearing	16 2 32
16 2 46	Trunnion Design Drawing	16 2 32
16 2 47	Rear Lock Assembly	16.2.32
16.2.17	Center Lock Jaws	16 2 34
16 2 49	Transverse Locks on Underside can be Disengaged	16.2.31
16 2 50	Wire Rone	16.2.35
16.2.51	Wire Rope Sockets and Fittings	16.2.36
16.2.52	Drums Wind Up the Up-Haul (Lifting) Ropes as	10.2.50
10.2.02	they Simultaneously Unwind the Down-Haul Rones.	16.2.37
16.2.53	Operator's House with Clear View of Traffic	101210 /
	Signals and Lane Gates	16.2.40
16.2.54	Traffic Control Gate	16.2.40
16.2.55	Navigational Light	16.2.41
16.2.56	Marine Two-Way Radio Console	16.2.42
16.2.57	Control Panel	16.2.45
16.2.58	Stress Reversals in Members	16.2.46
16.2.59	Concrete Bearing Areas	16.2.47
16.2.60	Pier Protection Systems – Dolphins and Fenders	16.2.47
16.2.61	Cracked Speed Reducer Housing	16.2.50
16.2.62	Leaking Speed Reducer	16.2.50
16.2.63	Hairline Crack Revealed on Shaft from Dve	
	Penetrant Test	16.2.51
16.2.64	Leaking Bearing	16.2.52
16.2.65	Open Switchboard	16.2.57
16.2.66	Hydraulic Power Specialists	16.2.59
Figure Nos.		Page Nos.
----------------	--	--------------
16.2.67	Example of Notes on Operating Machinery (Gears-	
	General)	16.2.61
16.2.68	Example of Notes on Operating Machinery (Gears-	
	Teeth)	16.2.62
16.2.69	Example of Notes on Operating Machinery	
	(Bearings)	16.2.63
16.2.70	Example of Notes on Operating Machinery	
	(Mechanical Components)	16.2.64
16.2.71	Example of Notes on Electrical Equipment (Motors).	16.2.65
16.2.72	Example of Notes on Electrical Equipment (Limit	
	Switch)	16.2.66
16.2.73	Example of Notes on Electrical Equipment (Megger	
	Insulation Test of the Submarine Cables)	16.2.67

Topic 16.3 Floating Bridges

16.3.1	Floating Bridge, SR 520 Evergreen Point Bridge,	
	Seattle, WA During Stormy Weather	16.3.1
16.3.2	Movable Bridge Section of Evergreen Point Bridge,	
	Seattle, WA	16.3.2
16.3.3	Elevated Section of Evergreen Point Bridge, Seattle,	
	WA	16.3.2
16.3.4	Brookfield, Vermont, Floating Bridge Constructed	
	from Timber	16.3.3
16.3.5	Concrete Pontoons Under Construction	16.3.4
16.3.6	Concrete Pontoons Transported for Hood Canal	
	Project	16.3.4
16.3.7	Continuous Pontoon-Type Structure	16.3.5
16.3.8	Separate Pontoon Type Structure	16.3.6
16.3.9	Bridge Constructed with Separate Pontoons	16.3.6
16.3.10	Cross-Section of Anchor Cable	16.3.7
16.3.11	Anchor Cable Saddle	16.3.7
16.3.12	Precast Concrete Fluke Style Anchor	16.3.8
16.3.13	Pile Anchor	16.3.9
16.3.14	Open-Cell Gravity Block Anchor	16.3.9
16.3.15	Solid Gravity Slab Anchor	16.3.11
16.3.16	Inspector Opening Pontoon Access Hatch	16.3.13
16.3.17	Sample Pontoon Inspection Plan	16.3.14
16.3.18	Frayed Cables Removed from a Floating Bridge	16.3.16
16.3.19	Typical View of Heavy Corrosion within Pontoon	
	Port	16.3.16

This page intentionally left blank.

Table of Contents

Chapter 1 Bridge Inspection Programs

1.1	Histor	y of the National Bridge Inspection Program	1.1.1
	1.1.1	Introduction	1.1.1
	1.1.2	History of the National Bridge Inspection Program Background The 1970's The 1980's The 1990's The 2000's	1.1.2 1.1.2 1.1.3 1.1.4 1.1.5 1.1.6
	1.1.3	Today's National Bridge Inspection Program FHWA Training Current FHWA Reference Material	1.1.6 1.1.7 1.1.10

Abbreviations Used in this Section

-	American Association of State Highway Officials (1921 to 1973)
-	American Association of State Highway and Transportation Officials (1973 to present)
-	Manual for Maintenance Inspection of Bridges
-	Bridge Inspector's Reference Manual
-	Bridge Management System
-	FHWA Recording and Coding Guide for the Structure Inventory and Appraisal of the Nation's Bridges
-	Department of Transportation
-	fracture critical member
-	Federal Highway Administration
-	Highway Bridge Replacement & Rehabilitation
-	Hydraulic Engineering Circular
-	Intermodal Surface Transportation Efficiency Act
-	Bridge Inspector's Training Manual 70
-	Bridge Inspector's Training Manual 90
-	maintenance, repair and rehabilitation
-	National Bridge Inventory
-	National Bridge Inspection Standards
-	National Cooperative Highway Research Program
-	nondestructive testing
-	National Highway Institute
-	National Highway System
-	National Institute for Certification in Engineering Technologies
-	Transportation Equity Act of the 21 st Century
-	Transportation Research Board
-	Technical Working Group

Chapter 1 Bridge Inspection Programs

Topic 1.1 History of the National Bridge Inspection Program

1 1	1	
1.1		

Introduction

In the years since the Federal Highway Administration's landmark publication, *Bridge Inspector's Training Manual 90 (Manual 90)*, bridge inspection and inventory programs of state and local governments have formed an important basis for formal bridge management programs. During the 1990's, the state DOT's implemented comprehensive bridge management systems, which rely heavily on accurate, consistent bridge inspection data.

This manual, the *Bridge Inspector's Reference Manual (BIRM)*, updates *Manual 90* and reflects over 20 years of change.

Advances in technology and construction have greatly enhanced current bridge design. However, the emergence of previously unknown problem areas and the escalating cost of replacing older bridges make it imperative that existing bridges be evaluated properly to be kept open and safe.

There are four letters that define the scope of bridge inspections in this country: NBIS, meaning National Bridge Inspection Standards. The National Bridge Inspection Standards (NBIS) are Federal regulations establishing requirements for:

- Inspection procedures
- Frequency of inspections
- Qualifications of personnel
- Inspection reports
- Maintenance of bridge inventory

The **National Bridge Inventory (NBI)** is the aggregation of structure inventory and appraisal data collected by each state to fulfill the requirements of NBIS.

To better understand the National Bridge Inventory Program (NBIP), it is helpful to review the development of the program.

CHAPTER 1: Bridge Inspection Programs TOPIC 1.1: History of the National Bridge Inspection Program



Figure 1.1.1 Number of Bridges Built since 1900

1.1.2 History of the National Bridge Inspection Program

Background

During the bridge construction boom of the 1950's and 1960's, little emphasis was placed on safety inspection and maintenance of bridges. This changed when the 2,235-foot Silver Bridge, at Point Pleasant, West Virginia, collapsed into the Ohio River on December 15, 1967, killing 46 people (see Figure 1.1.2).



Figure 1.1.2 Collapse of the Silver Bridge

This tragic collapse aroused national interest in the safety inspection and maintenance of bridges. The U.S. Congress was prompted to add a section to the "Federal Highway Act of 1968" which required the Secretary of Transportation to establish a national bridge inspection standard. The Secretary was also required to develop a program to train bridge inspectors.

The 1970'sThus, in 1971, the National Bridge Inspection Standards (NBIS) came into being.
The NBIS established national policy regarding:

- Inspection procedures
- Frequency of inspections
- Qualifications of personnel
- Inspection reports
- Maintenance of state bridge inventory

Three manuals were subsequently developed. These manuals were vital to the early success of the NBIS. The first manual was the Federal Highway Administration (FHWA) *Bridge Inspector's Training Manual 70 (Manual 70)*. This manual set the standard for inspector training.

The second manual was the American Association of State Highway Officials (AASHO) *Manual for Maintenance Inspection of Bridges*, released in 1970. This manual served as a standard to provide uniformity in the procedures and policies for determining the physical condition, maintenance needs and load capacity of highway bridges.

The third manual was the FHWA *Recording and Coding Guide for the Structure Inventory and Appraisal of the Nation's Bridges (Coding Guide)*, released in July 1972. It provided thorough and detailed guidance in evaluating and coding specific bridge data.

With the publication of *Manual 70*, the implementation of national standards and guidelines, the support of AASHO, and a newly available FHWA bridge inspector's training course for use in individual states, improved inventory and appraisal of the nation's bridges seemed inevitable. Several states began in-house training programs, and the 1970's looked promising. Maintenance and inspection problems associated with movable bridges were also addressed. In 1977, a supplement to *Manual 70*, the *Bridge Inspector's Manual for Movable Bridges*, was added.

However, the future was not to be trouble free. Two predominant concerns were identified during this period. One concern was that bridge repair and replacement needs far exceeded available funding. The other was that NBIS activity was limited to bridges on the Federal Aid highway systems. This resulted in little incentive for inspection and inventory of bridges not on Federal Aid highway systems.

These two concerns were addressed in the "Surface Transportation Assistance Act of 1978." This act provided badly needed funding for rehabilitation and new

construction and required that all public bridges over 20 feet in length be inspected and inventoried in accordance with the NBIS by December 31, 1980. Any bridge not inspected and inventoried in compliance with NBIS would be ineligible for funding from the special replacement program.

In 1978, the American Association of State Highway and Transportation Officials (AASHTO) revised their *Manual for Maintenance Inspection of Bridges (AASHTO Manual)*. In 1979, the NBIS and the FHWA *Coding Guide* were also revised. These publications, along with *Manual 70*, provided state agencies with definite guidelines for compliance with the NBIS.

The 1980'sThe National Bridge Inspection Program was now maturing and well positioned
for the coming decade. Two additional supplements to Manual 70 were published.
First, culverts became an area of interest after several tragic failures. The 1979
NBIS revisions also prompted increased interest in culverts. The Culvert
Inspection Manual was published July 1986. Then, an emerging national emphasis
on fatigue and fracture critical bridges was sharply focused by the collapse of
Connecticut's Mianus River Bridge in June 1983. Inspection of Fracture Critical
Bridge Members was published in September 1986. These manuals were the
products of ongoing research in these problem areas.

With the April 1987 collapse of New York's Schoharie Creek Bridge, national attention turned to underwater inspection. Of the over 593,000 bridges in the national inventory, approximately 86% were over waterways. The FHWA responded with *Scour at Bridges*, a technical advisory published in September 1988. This advisory provided guidance for developing and implementing a scour evaluation program for the:

- > Design of new bridges to resist damage resulting from scour
- Evaluation of existing bridges for vulnerability to scour
- Use of scour countermeasures
- > Improvement of the state-of-practice of estimating scour at bridges

Further documentation is available on this topic in the *Hydraulic Engineering Circular No. 18 (HEC-18)*.

In September 1988, the NBIS was modified, based on suggestions made in the "1987 Surface Transportation and Uniform Relocation Assistance Act," to require states to identify bridges with fracture critical details and establish special inspection procedures. The same requirements were made for bridges requiring underwater inspections. The NBIS revisions also allowed for adjustments in the frequency of inspections and the acceptance of National Institute for Certification in Engineering Technologies (NICET) Level III and IV certification for inspector qualifications.

In December 1988, the FHWA issued a revision to the *Coding Guide*. This time the revision would be one of major proportions, shaping the National Bridge Inspection Program for the next decade. The *Coding Guide* provided inspectors with additional direction in performing uniform and accurate bridge inspections.

The 1990'sThe 1990's was the decade for bridge management systems (BMS). Several states,
including New York, Pennsylvania, North Carolina, Alabama and Indiana, had
their own comprehensive bridge management systems.

In 1991, the FHWA sponsored the development of a bridge management system called "Pontis" which is derived from the Latin word for bridge. The Pontis system has sufficient flexibility to allow customization to any agency or organization responsible for maintaining a network of bridges.

Simultaneously, the National Cooperative Highway Research Program (NCHRP) of the Transportation Research Board (TRB) developed a BMS software called "Bridgit." Bridgit is primarily targeted to smaller bridge inventories or local highway systems.

As more and more bridge needs were identified, it became evident that needed funding for bridge maintenance, repair and rehabilitation (MR&R) far exceeded the available funding from federal and state sources. Even with the infusion of financial support provided by the Intermodal Surface Transportation Efficiency Act (ISTEA) of 1991, funding for bridge MR&R projects was difficult to obtain. This was due in part to the enormous demand from across the nation. An October 1993 revision to NBIS permitted bridge owners to request approval from FHWA of extended inspection cycles of up to forty-eight months for bridges meeting certain requirements.

In 1994, the American Association of State Highway and Transportation Officials (AASHTO) revised their *Manual for Condition Evaluation of Bridges (AASHTO Manual)*. In 1995, the FHWA *Coding Guide* was also revised. These publications, along with *Manual 90, Revised July 1995*, provided state agencies with continued definite guidelines for compliance with the NBIS and conducting bridge inspection.

Although later rescinded in the next transportation bill, the ISTEA legislation required that each state implement a comprehensive bridge management system by October 1995. This deadline represented a remarkable challenge since few states had previously implemented a BMS that could be considered to meet the definition of a comprehensive BMS. In fact, prior to the late 1980's, there were no existing management systems adaptable to the management of bridge programs nor was there any clear, accepted definition of key bridge management principles or objectives.

This flexibility in the system was the result of developmental input by a Technical Working Group (TWG) comprised of representatives from the FHWA, the Transportation Research Board (TRB) and the following six states: California, Minnesota, North Carolina, Tennessee, Vermont and Washington. The TWG provided guidance drawing on considerable experience in bridge management and engineering.

The National Highway System (NHS) Act of 1995 rescinded the requirement for bridge management systems. However, many of the states continued to implement the Pontis BMS.

The Transportation Equity Act of the 21st Century (TEA-21) was signed into law in June 1998. TEA-21 built on and improved the initiatives established in ISTEA and, as mentioned earlier, rescinded the mandatory BMS requirement.

The 2000'sIn 2002, Manual 90 was revised and updated as a part of a complete overhaul of
the FHWA Bridge Safety Inspection training program. The new manual was
named the Bridge Inspector's Reference Manual (BIRM) and incorporated all of
Manual 90. The BIRM also incorporates manual 70 Supplements for culvert
inspection and Fracture Critical Members. The BIRM was also updated in 2011.

On December 14, 2004, the revised NBIS regulation was published in the *Federal Register*. The updated NBIS took affect January 13, 2005. Implementation plans were to be developed by April 13, 2005 to be fully implemented by January 13, 2006.

The Safe, Accountable, Flexible, Efficient Transportation Equity Act: A Legacy for Users (SAFETEA-LU) was signed into law in August 2005. SAFETEA-LU represents the largest surface transportation investment in the Nation's history. SAFETEA-LU builds on and improves the initiatives established in ISTEA and TEA-21. Since being signed into law in August 2005, SAFETEA-LU has undergone several extensions past its original 2009 expiration, resulting in guaranteed funding until the end of Fiscal Year (FY) 2011. Multi-year legislation is expected to provide funding following FY 2011.

Over the years, varying amounts of federal funds have been spent on bridge projects, depending on the demands of the transportation infrastructure. Figure 1.1.3 illustrates the fluctuations in federal spending and shows current trends.



Figure 1.1.3 Federal Funding Levels (1979 – 2003)

1.1.3

Today's National Bridge Inspection Program

Much has been learned in the field of bridge inspection, and a national Bridge Inspection Training Program is now fully implemented. State and federal inspection efforts are more organized, better managed and much broader in scope. The technology used to inspect and evaluate bridge members and bridge materials has significantly improved.

Areas of emphasis in bridge inspection programs are changing and expanding as new problems become apparent, as newer bridge types become more common, and as these newer bridges age enough to have areas of concern. Guidelines for inspection ratings have been refined to increase uniformity and consistency of inspections. Data from bridge inspections has become critical input into a variety of analyses and decisions by state agencies and the Federal Highway Administration.

The NBIS has kept current with the field of bridge inspection. The 2005 National Bridge Inspection Standards appear in Appendix A. The standards are divided into the following sections:

- > Purpose
- > Applicability
- Definitions
- Bridge inspection organization
- Qualifications of personnel
- Inspection frequency
- Inspection procedures
- ➢ Inventory
- Reference manuals

The FHWA has made a considerable effort to make available to the nation's bridge inspectors the information and knowledge necessary to accurately and thoroughly inspect and evaluate the nation's bridges.

FHWA Training The FHWA has developed and now offers the following training courses relative to structure inspection through the National Highway Institute (NHI):

 "Engineering Concepts for Bridge Inspectors" (NHI Course Number FHWA-NHI-130054)

This one-week course is a pre-requisite for FHWA-NHI-130055 and presents engineering concepts, as well as inspection procedures and information about bridge types, bridge components, and bridge materials. The one-week course is for new inspectors with little or no practical bridge inspection experience.

"Introduction to Safety Inspection of In-Service Bridges" (NHI Course Number FHWA-NHI-130101)

This web-based course is another possible pre-requisite for FHWA-NHI-130055 and presents engineering concepts, as well as inspection procedures and information about bridge types, bridge components, and bridge materials. The course is for new inspectors with little or no practical bridge inspection experience.

"Safety Inspection of In-Service Bridges" (NHI Course Number FHWA-NHI-130055) This two-week course is for inspectors or engineers who perform or manage bridge inspections. Emphasis is on inspection applications and procedures. The uniform coding and rating of bridge elements and components is also an objective of the two-week course. A unique feature of this course allows for customization of the course content by the host agency. Some states use component rating based on NBIS while some states use element condition level evaluation. Lessons include critical findings identification and inspection of fracture critical members (FCM's), underwater inspection, culverts, field trips, case studies, and non-destructive evaluation. Several special bridge types may also be discussed at the host agency's request.

 "Bridge Inspection Refresher Training" (NHI Course Number FHWA-NHI-130053)

This three-day course provides a review of the National Bridge Inspection Standards (NBIS) and includes discussions on structure inventory items, structure types, and the appropriate codes for the Federal Structure, Inventory and Appraisal reporting. A three-and-a-half day option is also available, which includes additional case studies.

"Underwater Bridge Inspection" (NHI Course Number FHWA-NHI-130091)

This four or five-day course provides an overview of diving operations that will be useful to agency personnel responsible for managing underwater bridge inspections. This course also fulfills the requirement due to the latest changes of the National Bridge Inspection Standards, which require bridge inspection training for all divers conducting underwater inspections.

 "Underwater Bridge Repair Rehabilitation and Countermeasures" (NHI Course Number FHWA-NHI-130091A)

This two-day course provides training in techniques for selecting and executing repairs to below water bridge elements. The primary goal is to enable design engineers to select, design, and specify appropriate and durable repairs to below water bridge elements. The secondary goal of the course is to train staff in effective construction inspection of below water repairs.

 "Fracture Critical Inspection Techniques for Steel Bridges" (NHI Course Number FHWA-NHI-130078)

This three and one-half day course provides an understanding of fracture critical members (FCM's), FCM identification, failure mechanics and fatigue and fracture in metal. Emphasis is placed on inspection procedures and reporting of common FCM's and non-destructive evaluation (NDE) methods most often associated with steel highway bridges.

 "Bridge Inspection Non-Destructive Evaluation Showcase (BINS)" (NHI Course Number FHWA-NHI-130099) This one-day course allows bridge inspectors to identify the components of handheld NDE inspection tools and techniques, inspection strategies and NDE techniques. Inspection tools will include eddy current, ultrasonic, infrared thermography, impact echo, and ground penetrating radar.

 "Stream Stability and Scour at Highway Bridges" (NHI Course Number FHWA-NHI-135046)

> This three-day course provides training in the prevention of hydraulicrelated failures of highway bridges by identifying stream stability and scour problems at bridges and defining problems caused by stream instability and scour. The magnitude of scour at bridge piers and abutments and in the bridge reach will be estimated.

 "Stream Stability and Scour at Highway Bridges for Bridge Inspectors" (NHI Course Number FHWA-NHI-135047)

This one-day course concentrates on visual keys to detecting scour and stream instability problems. The course emphasizes inspection guidelines to complete the hydraulic and scour-related coding requirements of the National Bridge Inspection Standards (NBIS).

"Pontis Bridge Management" (NHI Course Number FHWA-NHI-134056)

This two and one-half day course covers the entering and editing of inspection data, developing a bridge preservation policy, performing bridge network level analyses, developing bridge projects, running Pontis and Infomaker reports, and refining Pontis results.

"Pontis Bridge Management and InfoMaker Module" (NHI Course Number FHWA-NHI-134056A)

This three and one-half day course covers the entering and editing of inspection data, developing a bridge preservation policy, performing bridge network level analyses, developing bridge projects, running Pontis and Infomaker reports, and refining Pontis results. It also includes an overview of InfoMaker 9.0 as it relates to Pontis. It covers those aspects most used by the Pontis users as well as the ability to query data, create new report libraries, modify existing Pontis structure list layout, and modify an existing Pontis report.

"Pontis Bridge Management InfoMaker Module" (NHI Course Number FHWA-NHI-134056B)

This one-day course provides an overview of InfoMaker 9.0 as it relates to Pontis. It covers those aspects most used by the Pontis users as well as the ability to query data, create new report libraries, modify existing Pontis structure list layout, and modify an existing Pontis report.

 "Inspection and Maintenance of Ancillary Highway Structures" (NHI Course Number FHWA-NHI-130087) CHAPTER 1: Bridge Inspection Programs TOPIC 1.1: History of the National Bridge Inspection Program

This two-day course provides training in the inspection and maintenance of ancillary structures, such as structural supports for highway signs, luminaries, and traffic signals. Its goal is to provide agencies with information to aid in establishing and conducting an inspection program in accordance with the FHWA "Guidelines for the Installation, Inspection, Maintenance, and Repair of Structural Supports for Highway Signs, Luminaries, and Traffic Signals".

"Inspection of Mechanically Stabilized Earth Walls and Reinforced Soil Slopes" (NHI Course Number FHWA-NHI-132080)

This three-day course is part of a series to develop a training and qualification/certification program for field inspectors. A partial list of lessons addressed in the course are MSE wall and RSS types and durability; construction methods and sequences; alignment control; methods of fill and compaction control; plans, specifications, and the geotechnical report; shop drawings; and safety.

Throughout all the expansions and improvements in bridge inspection programs and capabilities, one factor remains constant: the overriding importance of the inspector's ability to effectively inspect bridge components and materials and to make sound evaluations with accurate ratings. The validity of all analyses and decisions based on the inspection data is dependent on the quality and the reliability of the data collected in the field.

Across the nation, the duties, responsibilities, and qualifications of bridge inspectors vary widely. The two keys to a knowledgeable, effective inspection are training and experience in performing actual bridge inspections. Training of bridge inspectors has been, and will continue to be, an active process within state highway agencies for many years. This manual is designed to be an integral part of that training process.

- NBIS. Code of Federal Regulations. 23 Highways Part 650, Subpart C National Bridge Inspection Standards.
- AASHTO. LRFD Bridge Design Specifications, 5th Edition. Washington, D.C.: American Association of State Highway and Transportation Officials, with 2010 Interims.
- FHWA. Recording and Coding Guide for the Structure Inventory and Appraisal of the Nation's Bridges. Washington, D.C.: United States Department of Transportation, 1995, Errata Sheet 03/ 2004.
 http://www.fhwa.dot.gov/bridge/mtguide.pdf
- FHWA. Bridge Inspector's Reference Manual. Washington, D.C.: United States Department of Transportation, 2002, Revised 2006, 2011.
- AASHTO. Manual for Bridge Evaluation, Second Edition. Washington, D.C.: American Association of State Highway and Transportation Officials, 2011.
- AASHTO. Guide Manual for Bridge Element Inspection. Washington, D.C.: American Association of State Highway and Transportation Officials, 2011.

Current FHWA

Reference Material

Table of Contents

Chapter 1 Bridge Inspection Programs

1.2	Respor	nsibilities of the Bridge Inspector
	1.2.1	Introduction
	1.2.2	Responsibilities of the Bridge Inspector and Engineer1.2.1Maintain Public Safety and Confidence1.2.1Protect Public Investment1.2.2Provide Bridge Inspection Program Support1.2.2Maintain Accurate Bridge Records1.2.3Fulfill Legal Responsibilities1.2.4Current NBIS Requirements1.2.5
	1.2.3	Qualifications of Bridge Inspectors1.2.5Program Manager1.2.5Team Leader1.2.5Inspector Qualifications1.2.6
	1.2.4	Liabilities
	1.2.5	Quality Control and Quality Assurance

Abbreviations Used in this Section

AASHO	-	American Association of State Highway Officials (1921 to 1973)
AASHTO	-	American Association of State Highway and Transportation Officials (1973 to present)
AASHTO Manual	-	Manual for Maintenance Inspection of Bridges
BIRM	-	Bridge Inspector's Reference Manual
BMS	-	Bridge Management System
Coding Guide	-	FHWA Recording and Coding Guide for the Structure Inventory and Appraisal of the Nation's Bridges
DOT	-	Department of Transportation
FCM	-	fracture critical member
FHWA	-	Federal Highway Administration
HBRR	-	Highway Bridge Replacement & Rehabilitation
HEC	-	Hydraulic Engineering Circular
ISTEA	-	Intermodal Surface Transportation Efficiency Act
Manual 70	-	Bridge Inspector's Training Manual 70
Manual 90	-	Bridge Inspector's Training Manual 90
MR&R	-	maintenance, repair and rehabilitation
NBI	-	National Bridge Inventory
NBIS	-	National Bridge Inspection Standards
NCHRP	-	National Cooperative Highway Research Program
NDT	-	nondestructive testing
NHI	-	National Highway Institute
NHS	-	National Highway System
NICET	-	National Institute for Certification in Engineering Technologies
TEA-21	-	Transportation Equity Act of the 21 st Century
TRB	-	Transportation Research Board
TWG	-	Technical Working Group

Topic 1.2 Responsibilities of the Bridge Inspector

1.2.1			
Introduction	Bridge inspection has played, and continues to play, an increasingly important in providing a safe infrastructure for the United States. As the nation's brid continue to age and deteriorate, an accurate and thorough assessment of e bridge's condition is critical in maintaining a safe, functional and reliable high system.		
	This topic presents the responsibilities of the bridge inspector/engineer and qualifications for bridge inspection personnel.		
1.2.2			
Responsibilities of	There are five basic responsibilities of the bridge inspector and engineer:		
the Bridge	> Maintain public safety and confidence		
Inspector and	 Protect public investment 		
Engineer	Provide bridge inspection program support		
	Maintain accurate bridge records		
	Fulfill legal responsibilities		
Maintain Public Safety and Confidence	The primary responsibility of the bridge inspector is to maintain public safety and confidence. The general public travels the highways and bridges without hesitation. However, when a bridge fails, the public's confidence in the bridge system is violated (see Figure 1.2.1).		
	The engineer's role is:		
	> To incorporate design safety factors.		
	To provide cost-effective designs.		
	 To review and evaluate reports. Rate each bridge as to its safe load capacity 		
	Engineers include a margin of safety in their designs to compensate for variations in the quality of materials and unknowns in vehicular traffic loadings through the life of the structure. In older bridges, especially those designed prior to the use of computer software programs and modern design codes, margin of safety also compensated for a lack of precise calculations and construction loading conditions.		
	The inspector's role is:		
	> To provide thorough inspections identifying bridge conditions and defects.		
	➢ To prepare condition reports documenting deficiencies and alerting supervisors or engineers of any findings which might impact the safety of the roadway user or the integrity of the structure.		



Figure 1.2.1 Mianus Bridge Failure

Protect Public Investment Another responsibility is to protect public investment in bridges. Be on guard for minor problems that can be corrected before they lead to costly major repairs. Also, be able to recognize bridge elements that need repair in order to maintain bridge safety and avoid replacement costs.

The current funding available to rehabilitate and replace deficient bridges is not adequate to meet the needs. It is important that preservation activities be a part of the bridge program to extend the performance life of as many bridges as possible and minimize the need for costly repairs or replacement.

The engineer's role is:

> To continually upgrade design standards to promote longevity of bridge performance such as the implementation of high performance materials and better performing bridge joints.

The inspector's role is:

- To continually be on guard for minor problems that can become costly repairs.
- To recognize bridge components that need repair in order to maintain bridge safety and avoid the need for costly replacement.
- To make recommendations to close a bridge if necessary.

Subpart C of the National Bridge Inspection Standards (NBIS) of the *Code of Federal Regulations*, 23 Highways Part 650, mandates:

Purpose

Provide Bridge

Support

Inspection Program

- > Applicability
- > Definitions
- Bridge inspection organization

- Qualifications of personnel
- Inspection frequency
- Inspection procedures
- Inventory
- Reference manuals

Bridge Inspection Programs are funded by public tax dollars. Therefore, the bridge inspector is financially responsible to the public.

The "Surface Transportation Act of 1978" established the funding mechanism for providing Federal funds for bridge replacement. The Act also established criteria for bridge inspections and requirements for compliance with the NBIS.

The "Intermodal Surface Transportation Efficiency Act" (ISTEA) of 1991 and the Transportation Equity Act for the 21st Century (TEA-21) of 1998 establish funding mechanisms for tolled and free bridges for bridge maintenance, rehabilitation and replacement to adequately preserve the bridges and their safety to any user.

The Safe, Accountable, Flexible, Efficient Transportation Equity Act: A Legacy for Users (SAFETEA-LU) was signed into law in August 2005. SAFETEA-LU represents the largest surface transportation investment in the Nation's history. SAFETEA-LU builds on and improves the initiatives established in ISTEA and TEA-21. Since being signed into law in August 2005, SAFETEA-LU has undergone several extensions past its original 2009 expiration, resulting in guaranteed funding until the end of Fiscal Year (FY) 2011. Multi-year legislation is expected to provide funding following FY 2011. Information on SAFETEA-LU can be found on the FHWA website:

http://www.fhwa.dot.gov/safetealu/factsheets/bridge.htm

Maintain Accurate Bridge Records

There are three major reasons why accurate bridge records are required:

a. A structure history file facilitates the identification and/or monitoring of deficiencies.

For example, two bridge abutments are measured for tilt during several inspection cycles, and the results are as follows:

Year	Abutment A	Abutment B
2011	4-3/16"	3-1/2"
2009	4-3/16"	2-1/4"
2007	4-1/8"	1-1/8"
2005	4"	1"

Looking at year 2011 measurements only indicate that Abutment A has a more severe problem. However, examining the changes each year, it is noted that the movement of Abutment A is slowing and may have stopped, while Abutment B is changing at a faster pace each inspection cycle. At the rate it is moving, Abutment B probably surpasses Abutment A by the next inspection.

b. To identify and assess bridge deficiencies and to identify and assess bridge repair requirements. Be able to readily determine, from the records, what

repairs are needed as well as a good estimate of quantities. Maintain reports on the results of the bridge inspection together with notations of any action taken to address the findings of such inspections.

- c. To identify and assess minor bridge deficiencies, to identify and assess bridge maintenance needs, and preservation needs in a similar manner to the repair requirements. Maintain relevant maintenance and inspection data to allow assessment of current bridge condition.
- d. To be able to quickly obtain pertinent structure information to respond to emergency events such as fire on or below the structure, severe flooding, and navigational or vehicular collision.
- e. To maintain load carrying capacity to facilitate the routing of overweight/over-height vehicles.

To ensure accurate bridge records, proper record keeping needs to be maintained. Develop a system to review bridge data and evaluate quality of bridge inspections. Bridge files are to be prepared as described in the *AASHTO Manual for Bridge Evaluation*. Record the findings and results of bridge inspections on standard State or Federal agency forms.

Fulfill LegalA bridge inspection report is a legal document. Make descriptions specific,
detailed, quantitative (where possible), and complete. Do not use vague
adjectives such as good, fair, poor, and general deterioration, without concise
descriptions to back them up. To say "the bridge is OK" is just not good enough.

Example of inspection descriptions:

Bad description: "Fair beams"

Good description: "Reinforced concrete tee-beams are in fair condition with light scaling on bottom flanges of Beams B and D for their full length."

Bad description: "Deck in poor condition"

Good description: "Deck in poor condition with spalls covering 50% of the top surface area of the deck as indicated indicated on field sketch, see Figure 42."

Bad description: "The bridge is dangerous"

Good description: "Section loss exists on Girder G5 at 10 feet north of centerline of bearing at Pier 1. Original flange thickness 1.5 inches. Measured thickness 0.991 inches."

Include phrases such as "no other apparent defects" or "no other defects observed" in any visual assessment.

Do not alter original inspection notes without consultation with the inspector who wrote the notes.

A bridge inspection report implies that the inspection was performed in accordance with the National Bridge Inspection Standards, unless specifically stated otherwise in the report. Use the proper equipment, methods, and qualified. If the inspection is a special or interim inspection, explained explicitly in the report.

Current NBIS Requirements The National Bridge Inspection Standards (NBIS) are regulations that were first established in 1971 to set national requirements regarding bridge inspection frequency, inspector qualifications, report formats, and inspection and rating procedures.

The NBIS can be found in the Code of Federal Regulations, Part 65, Title 23, Subpart C which is on the Bridge Technology site located on the FHWA website:

http://www.fhwa.dot.gov/bridge/nbis.htm

The NBIS set minimum, nationwide requirements. States and other owner agencies can establish additional or more stringent requirements.

1.2.3

Qualifications of Bridge Inspectors	The NBIS are very specific with regard to the qualifications of bridge inspectors. The <i>Code of Federal Regulations</i> , Title 23, Part 650, Subpart C, Section 650.309, (23 <i>CFR</i> 650.309), lists the qualifications of personnel for the National Bridge Inspection Standards (Appendix B of this Manual). These are minimum standards; therefore, state or local highway agencies can implement higher requirements.		
Program Manager	The program for bridge insp follows:	manager is in charge of the organizational unit that has responsibility pection, reporting, and inventory. The minimum qualifications are as	
	1)	Be a registered Professional Engineer, or have ten years bridge inspection experience; and	
	2)	Successfully complete a Federal Highway Administration (FHWA) approved comprehensive bridge inspection training course.	
Team Leader	The team lean inspections of inspection. N initial, routine alternative wa	ader is responsible for planning, preparing, and performing the of individual bridges as well as the day-to-day aspects of the NBIS calls for a team leader to be present at all times during each e, in-depth, fracture critical and underwater inspection. There are five anys to qualify as a team leader:	
	1)	Have the qualifications specified for the Program Manager: or	
	2)	Have five years bridge inspection experience and have successfully completed an FHWA-approved comprehensive bridge inspection training course; or	
	3)	Be certified as a Level III or IV Bridge Safety Inspector under the National Society of Professional Engineer's program for National Certification in Engineering Technologies (NICET) and have successfully completed an FHWA-approved comprehensive bridge inspection training course, or	
	4)	Have the following:	
		 A bachelor's degree in engineering from a college or university accredited by or determined as substantially equivalent by the Accreditation Board for Engineering and Technology; 	

- Successfully passed the National Council of Examiners for Engineering and Surveying Fundamentals of Engineering examination;
- iii) Two years of bridge inspection experience; and
- iv) Successfully completed an FHWA-approved comprehensive bridge inspection training course, or
- 5) Have the following:
 - An associate's degree in engineering or engineering technology from a college or university accredited by or determined as substantially equivalent by the Accreditation Board for Engineering and Technology;
 - ii) Four years of bridge inspection experience; and
 - iii) Successfully completed an FHWA-approved comprehensive bridge inspection training course.

Inspector Qualifications There are no specific federal guidelines for bridge inspectors. The main responsibility of a bridge inspector is to assist the team leader in day-to-day aspects of the inspection. Training is not required but it is recommended for non-team leaders. Any technical background is obtained through education and hands-on experience enables the inspector to successfully complete the tasks at hand. The goal is for the inspector to learn the correct inspection methods and to evaluate bridge components and elements consistently.

1.2.4

Liabilities

The dictionary defines tort as "a wrongful act for which a civil action lie except one involving a breach of contract."

In the event of negligence in carrying out the basic responsibilities described above, an individual, including department heads, engineers, and inspectors, is subject to personal liability. Strive to be as objective and complete as possible. Accidents that result in litigation are generally related, but not necessarily limited, to the following:

- Deficient safety features
- Failed members
- Failed substructure elements
- Failed joints or decks
- Potholes or other hazards to the traveling public
- Improper or deficient load posting

Anything said or written in the bridge file could be used in litigation cases. In litigation involving a bridge, the inspection notes and reports may be used as evidence. A subjective report may have negative consequences for the highway agency involved in lawsuits involving bridges. The report scrutinized to determine if conditions are documented thoroughly and for the "proper" reasons. Therefore, be as objective and complete as possible. State if something could not be inspected and the reason it was not inspected.

Example of liabilities:

In a recent case, a consulting firm was found liable for negligent inspection practices. A tractor-trailer hit a large hole in a bridge deck, swerved, went through the bridge railing, and fell 30 feet to the ground. Ten years prior to the accident, the consulting firm had noted severe deterioration of the deck and had recommended tests to determine the need for replacement. Two years prior to the accident, their annual inspection report did not show the deterioration or recommend repairs. One year before the accident, inspectors from the consultant checked 345 bridges in five days, including the bridge on which the accident occurred. The court found that the consulting firm had been negligent in its inspection, and assessed the firm 75% of the ensuing settlement.

In another case, four cars drove into a hole 12 feet deep and 30 feet across during the night. Five people were killed and four were injured. The hole was the result of a collapse of a multi-plate arch. Six lawsuits were filed and, defendants included the county, the county engineer, the manufacturer, the supplier, and the consulting engineers who inspected the arch each year. The arch was built and backfilled, with mostly clay, by a county maintenance crew 16 years prior to the accident. Three years later, the county engineer found movement of three to four inches at one headwall. The manufacturer sent an inspector, who determined that the problem was backfill-related and recommended periodic measurements. These measurements were done once, but the arch was described as "in good condition" or "in good condition with housekeeping necessary" on subsequent inspections. Inspection reports documented a six inch gap between the steel plate and the headwall. A contractor examined the arch at the county engineer's request to provide a proposal for shoring. The county engineer discussed the proposal with the consulting engineers a month before the accident. A total of 13 inspections were conducted on the structure. An engineering report accuses the county engineer of poor engineering practice.

1.2.5

Quality Control and Quality Assurance The NBIS requires Quality Control (QC) and Quality Assurance (QA) procedures to maintain a high degree of accuracy and consistency in the highway bridge inspection program. Accuracy and consistency are important since the bridge inspection process is the foundation to the bridge management systems. FHWA has developed a recommended framework for a bridge inspection QC/QA program (see Topic 1.3).

CHAPTER 1: Bridge Inspection Programs TOPIC 1.2: Responsibilities of the Bridge Inspector

This page intentionally left blank

Table of Contents

Chapter 1 Bridge Inspection Programs

1.3	Quality	v Control and Quality Assurance
	1.3.1	Introduction1.3.1
	1.3.2	Quality Control 1.3.1
	1.3.3	Quality Assurance
	1.3.4	Quality Control and Quality Assurance Framework1.3.2

This page intentionally left blank

Topic 1.3 Quality Control and Quality Assurance

1.3.1	
Introduction	Title 23, <i>Code of Federal Regulations (CFR)</i> , Part 650, Subpart C, Section 313, paragraph (g), Quality Control and Quality Assurance, requires each state to assure that systematic Quality Control (QC) and Quality Assurance (QA) procedures are being used to maintain a high degree of accuracy and consistency in their inspection program. The FHWA has developed a recommended framework for a bridge inspection QC/QA program to assist bridge owners in developing their QC / QA programs.
	Accuracy and consistency of the data is important since the bridge inspection process is the foundation of the entire bridge management operation and bridge management systems. Information obtained during the inspection is used for determining needed maintenance and repairs, for prioritizing rehabilitations and replacements, for allocating resources, and for evaluating and improving design for new bridges. The accuracy and consistency of the inspection and documentation is vital because it not only impacts programming and funding appropriations, it also affects public safety.
1.3.2	
Quality Control	Quality Control (QC) is the establishment and enforcement of procedures that are intended to maintain the quality of the inspection at or above a specific level. If an inspection program is decentralized, the state program manager is still responsible for QC.
1.3.3	
Quality Assurance	Quality Assurance (QA) is the use of sampling and other measures to assure the adequacy of quality control procedures in order to verify or measure the quality level of the entire bridge inspection and load rating program. This is accomplished by the re-inspection of a sample of bridges by an independent inspection team. For decentralized state inspections or delegated inspection programs, the QA program can be performed by the central staff or their agent (e.g., consultants). If the inspections are centralized within the state, consultants or a division are to perform the QA program separate and independent of the inspection state organization.
	The quality of the inspection and reports rests primarily with the inspection team leaders and team members and their knowledge and professionalism in developing a quality product. A QC/QA program is a means by which periodic and independent inspections, reviews, and evaluations are performed in order to provide feedback concerning the quality and uniformity of the state's or agency's inspection program. The feedback is then used to enhance the inspection program through improved inspection processes and procedures, training, and quality of the inspection report.

1.3.4

Quality Control and The FHWA has developed the following recommended framework for a bridge inspection QC/QA program.

Framework

A. Documentation of QC/QA Program:

- 1. Develop, document, and maintain a bridge inspection manual that contains Quality Control/Quality Assurance (QC/QA) procedures in accordance with this recommended framework.
- 2. Elaborate on the purpose and benefits of the QC/QA program.
- 3. Provide appropriate definitions.

B. Quality Control (QC) Procedures

- 1. Define and document QC roles and responsibilities.
- 2. Document qualifications required for Program Manager, Team Leader, Team Member, Load Rater and Underwater Bridge Inspection Diver.
- 3. Document process for tracking how qualifications are met, including:
 - a. Years and type of experience.
 - b. Training completed.
 - c. Certifications/registrations.
- 4. Document required refresher training, including:
 - a. NHI training courses, other specialized training courses, and/or periodic meetings.
 - b. Define refresher training content, frequency, and method of delivery.
- 5. Document special skills, training, and equipment needs for specific types of inspections.
- 6. Document procedures for review and validation of inspection reports and data.
- 7. Document procedures for identification and resolution of data errors, omissions and/or changes.

C. Quality Assurance (QA) Procedures

- 1. Define and document QA roles and responsibilities.
- 2. Document procedures for conducting office and field QA reviews, including:

- a. Procedures for maintaining, documenting, and sharing review results; including an annual report.
- b. Establish review frequency parameters. Parameters should include:
 - i. Recommended review frequency for districts/units to be reviewed (e.g. review each district once every four years). Or establish number of districts/units to be reviewed annually.
 - ii. Recommended number of bridges to review.
- c. Procedures and sampling parameters for selecting bridges to review. Procedure should consider:
 - i. Whether the bridge is or is not posted.
 - ii. Bridge's deficiency status.
 - iii. Whether the bridge is programmed for rehabilitation or replacement.
 - iv. Whether the bridge has had critical findings and the status of any follow-up action.
 - v. Bridges with unusual changes in condition ratings (e.g. more than 1 appraisal rating change from previous inspection).
 - vi. Bridges that require special inspections (underwater, fracture critical, other special).
 - vii. Location of bridge.
- d. Procedures for reviewing current inspection report, bridge file, and load rating.
- e. Procedures to validate qualifications of inspector and load rater.
- f. Define "out-of-tolerance" for condition rating and load rating. (e.g. rating of +/- 1 or load ratings that differ by more than 15%)
- g. Checklists covering typical items to review as part of QA procedures.
 - i. Bridge file.
 - ii. Field inspection.
 - iii. Load rating analysis.
- h. Others.

- 3. Document disqualification procedures for team leaders and consultant inspection firms that have continued record of poor performance.
- 4. Document re-qualification procedures for previously disqualified team leaders and consultant inspection firms that demonstrate they have acceptable performance.
- 5. Document procedures for conducting inspections on a "control" bridge.
- 6. Document procedures to validate the QC procedures.

Examples of Commendable State practices and additional resources regarding QC/QA programs are available at the following link: http://www.fhwa.dot.gov/bridge/nbis/nbisframework.cfm

Table of Contents

Chapter 2 Safety Fundamentals for Bridge Inspectors

2.1	Duties of the Bridge Inspection Team 2.1.			
	2.1.1	Introduction		
	2.1.2	Duties of the Bridge Inspection Team 2.1.2		
	2.1.3	Planning the Inspection		
	2.1.4	Preparing for Inspection2.1.2Review Bridge Structure File2.1.3Identify Components and Elements2.1.3Deck Element Numbering System2.1.4Superstructure Element Numbering System2.1.4Substructure Element Numbering System2.1.5AASHTO Bridge Elements2.1.5Develop Inspection Sequence2.1.5Prepare and Organize Notes, Forms, and Sketches2.1.6Arrange for Temporary Traffic Control2.1.7Time Requirements2.1.7Peak Travel Times2.1.8Set-up Time2.1.8Access2.1.8Weather2.1.8Safety Precautions2.1.9Tools2.1.9Subcontract Special Activities2.1.9		
	2.1.5	Performing the Inspection2.1.9General Inspection Procedures2.1.9Approaches and Decks2.1.10Superstructures2.1.11Substructures2.1.11Culverts2.1.11Waterways2.1.11Inspection of Bridge Elements2.1.12Timber Inspection2.1.13Metal Inspection: Steel, Iron and Others2.1.13Fiber Reinforced Polymer Inspection2.1.13Critical Findings2.1.14		

2.1.6	Preparing the Report	
2.1.7	Identifying Items for Preservation and Follow-up f Findings	or Critical 2.1.14
2.1.8	Types of Bridge Inspection Initial (Inventory) Routine (Periodic) Damage In-Depth Fracture Critical Underwater Special (Interim)	

Chapter 2 Safety Fundamentals for Bridge Inspectors

Topic 2.1 Duties of the Bridge Inspection Team

2.1.1

Introduction

Bridge inspection plays an important role in providing a safe infrastructure for the nation. As the nation's bridges continue to age and deteriorate, an accurate and thorough assessment of each bridge's condition is critical in maintaining a dependable highway system.

There are seven basic types of inspection:

- Initial (inventory)
- Routine (periodic)
- ➢ Damage
- In-depth
- ➢ Fracture critical
- > Underwater
- Special (interim)

These inspection types are presented in Article 4.2 of the AASHTO *Manual for Bridge Evaluation*. Although this topic is organized for "in-depth" inspections, it applies to any inspection type. However, the amount of time and effort required for performing each duty vary with the type of inspection performed.

This topic presents the duties of the bridge inspection team. It also describes how the inspection team can prepare for the inspection and some of the major inspection procedures. For some duties, the inspection program manager may be involved.

2.1.2		
Duties of the Bridge	There are five basic duties of the bridge inspection team:	
Inspection Team	Planning the inspection	
	Preparing for the inspection	
	Performing the inspection	
	Preparing the report	
	Identifying items for repairs and maintenance	
	Communicate the need for immediate follow-up for critical findings	
2.1.3		
Planning the Inspection	Planning is necessary for a safe, efficient, cost-effective inspection effort which results in a thorough and complete inspection of in-service bridges.	
	Basic activities include:	
	Determination of the type of inspection	
	Selection of the inspection team, which includes a qualified team leader on site for all initial, routine, in-depth, fracture critical and underwater inspections. Although not required by NBIS, it is a good practice to provide a team leader for damage and special inspections.	
	Evaluation of required activities (e.g., nondestructive evaluation, traffic control including use of flaggers, utilities, confined spaces, permits, hazardous materials such as pigeon droppings, lead paint and asbestos removal, etc.)	
	Establishment of a schedule which includes the inspection duration	
2.1.4		
Preparing for Inspection	Preparation measures needed prior to the inspection include organizing the prior tools and equipment, reviewing the bridge structure files, and locating plans for structure. The success of the on-site field inspection is largely dependent or effort spent in preparing for the inspection. The major preparation activinclude:	
	Review the bridge structure file	
	Identify the components and elements	
	Develop an inspection sequence	
	Prepare and organize notes, forms, and sketches	
	 Arrange for temporary traffic control 	
	Arrange staging areas and access locations	
	Reviewing safety precautions	
	 Organizing tools and equipment 	
	Arranging for subcontracting special activities	
	 Account for other special considerations 	

Review the Bridge The first step in preparing for a bridge inspection is to review the available sources of information about the bridge, such as: **Structure File**

- \triangleright Plans, including construction plans, shop and working drawings, and asbuilt drawings
- Specifications \geq
- Correspondence \triangleright
- Photographs \geq
- Materials and tests, including material certification, material test data, and \geq load test data
- \geq Maintenance and repair history
- Coating history \geq
- \triangleright Accident records
- ≻ Posting
- \triangleright Permit loads
- \triangleright Flood and scour data
- \triangleright Traffic data
- Inspection history \geq
- \geq Inspection requirements
- \geq Structure Inventory and Appraisal sheets
- \geq Inventories and inspections
- \triangleright Rating records

Each of these sections of the bridge structure file is presented in detail in Topic 4.4.2.

Elements

Identify Components and Another important activity in preparing for the inspection is to establish the structure orientation, as well as a system for identifying the various components and elements of the bridge (see Figure 2.1.1). If drawings or previous inspection reports are available, use the same identification system during the inspection as those used in these sources.



Figure 2.1.1 Sample Bridge Numbering Sequence

Establish an identification system if there are no previous records available. The numbering system presented in this topic is one possible system, but some states may use a different numbering system.

This route direction information can be used to identify the location of the bridge.

Route direction would be north, south, east or west. Mile markers, stationing or segments are the locations along the route. Location of the bridge can be identified by the route direction along with mile marker, stationing or segment information. The route direction can be determined based on mile markers, stationing, or segments, and use this direction to identify the location of the bridge.

Deck Element Numbering System

The deck sections (between construction joints), expansion joints, railing, parapets, and light standards are included in the deck element numbering system. Number these elements consecutively, from the beginning to the end of the bridge.

Superstructure Element Numbering System

The spans, the beams, and, in the case of a truss or arch, the panel points are included in the superstructure element numbering system. Number the spans consecutively, with Span 1 located at the beginning of the bridge. Multiple beams are to be numbered consecutively from left to right facing in the route direction. Similar to spans, floorbeams are also numbered consecutively from the beginning of the bridge, with the first floorbeam labeled as Floorbeam 0. This coordinates the floorbeam and the bay numbers such that a given floorbeam number is located at the end of its corresponding bay.

For trusses, number the panels similarly to the floorbeams, beginning with Panel Point 0. Label both the upstream and downstream trusses. Points in the same vertical line have the same number. If there is no lower panel point in a particular vertical line, the numbers of the lower chord skip a number (see Figure 2.1.2). Some design plans number to midspan on the truss and then number backwards to zero using prime numbers (U9'). However, this numbering system is not recommended for field inspection use since the prime designations in the field notes may be obscured by dirt.



Figure 2.1.2 Sample Truss Numbering Scheme
Substructure Element Numbering System

	The abutments and the piers are included in the substructure element numberin system. Abutment 1 is located at the beginning of the bridge, and Abutment 2 i located at the end. Number the piers consecutively, with Pier 1 located closest t the beginning of the bridge (see Figure 2.1.2). Alternatively, the substructure unit may be numbered consecutively without noting abutments or piers.			
	AASHTO Bridge Elements			
	The AASHTO Guide Manual for Bridge Element Inspection provides comprehensive set of bridge elements, designed to be flexible in nature to satisf needs of all agencies. This set of elements capture the components necessary for any agency to manage the aspects of the bridge inventory and allows the ful utilization of a Bridge Management System (BMS).			
	There are two different element types included in the element set which are identified as National Bridge Elements (NBEs) or Bridge Management Elements (BMEs). These two element sets combined comprise the full AASHTO element set.			
Develop Inspection Sequence	An inspection normally begins with the deck and superstructure elements and proceeds to the substructure. However, there are many factors to be considered when planning a sequence of inspection for a bridge, including:			
	> Type of bridge			
	 Condition of the bridge components 			
	 Overall condition 			
	Inspection agency requirements			
	Size and complexity of the bridge			
	Traffic conditions			
	Special considerations			
	A sample inspection sequence for a bridge of average length and complexity is presented in Figure 2.1.3. While developing an inspection sequence is important, it is of value only if following it ensures a safe, complete and thorough inspection of the bridge.			

1) Roadway Elements

- Approach roadways
- Traffic safety features
- ➢ General alignment
- > Approach alignment
- > Deflections
- > Settlement

2) Deck Elements

- Bridge deck: top and bottom
- Expansion joints
- Sidewalks and railings
- Drainage
- > Signing
- Electrical-lighting
- Barriers, gates, and other traffic control devices

4) Substructure Elements

- > Abutments
- > Piers
- ➢ Footings
- > Piles
- Curtain walls
- Skewbacks (arches)
- Slope protection

5) Channel and Waterway Elements

- Channel profile and alignment
- Channel streambed
- > Channel embankment
- > Channel embankment protection
- > Hydraulic opening Fenders
- ➢ Water depth scales
- Navigational lights and aids
- > Dolphins
- Hydraulic control devices

3) Superstructure Elements

- Primary load-carrying members
- Secondary members and bracings
- Utilities and their attachments
- Anchorages
- ➢ Bearings

Figure 2.1.3 Sample Inspection Sequence

Prepare and Organize Notes, Forms, and Sketches	Preparing notes, forms, and sketches prior to the on-site inspection reduces work in the field. Obtain copies of the agency's standard inspection form for use in recordkeeping and as a checklist to ensure that the condition of all elements is noted.
	Create copies of sketches from previous inspection reports so that defects previously documented can simply be updated. Preparing extra copies provides a contingency for sheets that may be lost or damaged in the field.
	If previous inspection sketches or design drawings are not available, then pre- made, generic sketches may be used for repetitive features or members. Possible applications of this timesaving method include deck sections, floor systems, bracing members, abutments, piers, and retaining walls. Numbered, pre-made sketches and forms can also provide a quality control check on work completed.
Arrange for Temporary Traffic Control	Bridge inspection, like construction and maintenance activities on bridges, often presents motorists with unexpected and unusual situations (see Figure 2.1.4). Most state agencies have adopted the Federal <i>Manual on Uniform Traffic Control Devices for Streets and Highways (MUTCD)</i> . Some state and local jurisdictions, however, issue their own manuals. When working in an area exposed to traffic, check and follow the governing standards. These standards prescribe the minimum methods for a number of typical applications and the proper use of

standard traffic control devices, such as cones, signs, and flashing arrow boards.



Figure 2.1.4Temporary Traffic Control Operation

Principles and methods, which enhance the safety of motorists and bridge inspectors in work areas, include the following:

- Traffic safety is a high priority element on every bridge inspection project where the inspectors' activities are exposed to traffic or likely to affect normal traffic movements.
- Route traffic through work areas with geometrics and traffic control devices comparable to those employed for other highway situations.
- Inhibit traffic and pedestrian movement as little as practicable.
- Guide approaching motorists in a clear and positive manner throughout the bridge inspection site.
- On long duration inspections, perform routine inspection of temporary traffic control devices.
- Adequately train personnel responsible for the performance of temporary traffic control operations.

In addition, schedules may have to be adjusted to accommodate temporary traffic control needs. For example, the number of lanes that can be closed at one time may require conducting the inspection operation with less than optimum efficiency. While it might be most efficient to inspect a floor system from left to right, traffic control may dictate working full length, a few beams at a time. Some agencies require inspections to be performed during low tow traffic (i.e. at night).

Special Considerations Time Requirements

The total time required to complete an inspection can vary from what may be documented on a previous inspection report or separately in the bridge file due to the various tasks for completing the inspection. Breaking down and recording the time to complete the various tasks (office preparation, travel, on-site, report preparation) separately benefits future planning and preparation efforts. Break down the inspection time requirements in to office preparation, travel time, field time, and report preparation. The overall condition of the bridge plays a major role in determining how long an inspection takes. Previous inspection reports provide an indication of the bridge's overall condition. It generally takes more time to inspect and document a deteriorated element (e.g., measuring, sketching, and photographing) than it does to simply observe and document that an element is in good condition.

Peak Travel Times

In populated areas, an inspection requiring traffic restrictions may be limited to certain hours of the day, such as 10:00 AM to 2:00 PM. Some days may be banned for inspection work altogether. Actual inspection time may be less than a 40-hour work week in these situations, so adjust the schedules accordingly.

Set-up Time

Consider set-up time both before and during the inspection. For example, rigging efforts may require several days before the inspectors arrive on the site. Also, other equipment, such as compressors and cleaning equipment may require daily set-up time. Provide adequate time in the schedule for set-up and take-down time requirements. Also, consider the time to install and remove temporary traffic control devices.

Access

Consider access requirements when preparing for an inspection. Bridge members may be very similar to each other, but they may require different amounts of time to gain access to them. For example, it may take longer to maneuver a lift device to gain access to a floor system near utility lines than for one that is free of obstructions. On some structures, access hatches may need to be opened to gain access to a portion of the bridge.

Weather

Adverse weather conditions may not halt an inspection entirely, but may play a significant role in the inspection process. During adverse weather conditions, avoid climbing on the bridge structure. An increased awareness of safety hazards is required, and keeping notes dry can be difficult. During seasons of poor weather, adopt a less aggressive schedule than during the good weather months.

Safety Precautions

While completing the inspection in a timely and efficient manner, the importance of taking safety precautions cannot be overlooked. Review general safety guidelines for inspection and any agency or bridge specific safety precautions such as for hazardous material and confined space entry. Confined space entry methods are in accordance with OSHA and the owners' requirements. For climbing inspections, the three basic requirements covered in topic 2.2.5 for safe climbing are to be followed. For additional information about safety precautions, refer to Topic 2.2.5.

Permits

When inspecting a bridge crossing a railroad, obtain an access permit before proceeding with the field inspection. Also obtain a permit when inspecting bridges passing over navigable waterways. Environmental permits and permits to work around endangered species may be required for some bridges and bridge sites.

Tools

To perform a complete and accurate inspection, use the proper tools and equipment. Bridge location and type are two main factors in determining required tools and equipment. Refer to Topic 2.4 for a complete list of inspection tools and equipment.

Subcontract Special Activities

Give consideration to time requirements when special activities are scheduled. These activities may include one or more of the following:

- Maintenance and protection of traffic (M.P.T.)
- Access, including rigging, inspection vehicle(s), or a combination there of
- Coordination with various railroads, including obtaining the services of railroad flagmen
- Non-destructive evaluation/testing

2.1.5

2.1.3			
Performing the Inspection	This duty is the on-site work of accessing and examining bridge components and waterway, if present.		
	Perform inspections in accordance with the National Bridge Inspection Standards (NBIS) and AASHTO Manual for Bridge Evaluation (MBE).		
	Basic activities include:		
	 Visual examination of bridge components 		
	Physical examination of bridge components		
	 Evaluation of bridge components 		
	Examination and evaluation of the waterway beneath the structure, if any, and approach roadway geometry		
General Inspection Procedures	Duties associated with the inspection include maintaining the proper structure orientation and member numbering system, and following proper inspection procedures.		
	The procedures used to inspect a bridge depend largely on the bridge type, the materials used, and the general condition of the bridge. Therefore, be families with the basic inspection procedures for a wide variety of bridges.		
	A first step in the inspection procedure is to establish the orientation of the site and of the bridge. Include the compass directions, the direction of waterway flow, and the direction of the inventory route in the orientation. Also record inspection team members, air temperature, weather conditions, and time.		

After the site orientation has been established, the inspector is ready to begin the on-site inspection. Be careful and attentive to the work at hand, and do not overlook any portion of the bridge. Give special attention to those portions that are most critical to the structural integrity of the bridge. (Refer to Topic 6.4 for a description of fracture critical members in steel bridges.)

Combine the prudence used during the inspection with thorough and complete recordkeeping. Careful and attentive observations are to be made, and record every deficiency. A very careful inspection is worth no more than the records kept during that inspection.

Place numbers or letters on the bridge by using crayon or paint to identify and code components and elements of the structure. The purpose of these marks is to keep track of the inspector's location and to guard against overlooking any portion of the structure.

Note the general approach roadway alignment, and sight along the railing and edge of the deck or girder to detect any misalignment or settlement.

Approaches and Decks

Check the approach pavement for unevenness, settlement, or roughness. Also check the condition of the shoulders, slopes, drainage, and approach guardrail.

Examine the deck and any sidewalks for various deficiencies, noting size, type, extent, and location of each deficiency. Reference the location using the centerline or curb line, the span number, and the distance from a specific pier or joint.

Examine the expansion joints for sufficient clearance and for adequate seal. Record the width of the joint opening at both curb lines, noting the air temperature and the general weather conditions at the time of the inspection.

Finally, check that safety features, signs (load restrictions), and lighting are present, and note their condition.

Superstructures

Inspect the superstructure thoroughly, since the failure of a primary load-carrying member could result in the collapse of the bridge. The primary method of bridge inspection is visual, requiring the removal of dirt, leaves, animal waste, and debris to allow close observation and evaluation of the primary load-carrying members. The most common forms of primary load-carrying members are:

- Beams and girders
- Floorbeams and stringers
- Trusses
- Cables (suspension, stay, suspender)
- Eyebar chains
- Arches
- ➤ Frames
- Pins and hanger assemblies

Bearings

Inspect the bearings thoroughly, since they provide the critical link between the superstructure and the substructure. The primary method of bearing inspection is a visual inspection, which requires removing dirt, leaves, animal waste, and debris to allow close observation and evaluation of the bearings. Record the difference between the rocker tilt and a fixed reference line, noting the direction of tilt, the air or bearing material temperature, and the general weather conditions at the time of the inspection.

Substructures

The substructure, which supports the superstructure, is made up of abutments, piers, and bents. If "design" or "as-built" plans are available, compare the dimensions of the substructure units with those presented on the plans. Since the primary method of bridge inspection is visual, remove the dirt, leaves, animal waste, and debris to allow close observation and evaluation. Check the substructure units for settlement by sighting along the superstructure and noting any tilting of vertical faces. In conjunction with the scour inspection of the waterway, check the substructure units for undermining, noting both its extent and location.

Culverts

Inspect culverts regularly to identify any potential safety problems and maintenance needs. Examine the culvert for various deficiencies, noting size, type, extent, and location of each deficiency. Reference the location using the centerline. In addition to the inspection of the culvert and its components, look for high-water marks, changes in drainage area, scour, and settlement of the roadway.

Waterways

Waterways are dynamic in nature, with their volume of flow and their path continually changing. Therefore, carefully inspect bridges passing over waterways for the effects of these changes.

Maintain a historical record of the channel profile and cross-sections. Record and compare current measures to initial (base line) measures, noting any meandering of the channel both upstream and downstream. Report any skew or improper location of the piers or abutments relative to the stream flow.

Scour is the removal of material from the streambed or streambank as a result of the erosive action of streamflow. Scour is the primary concern when evaluating the effects of waterways on bridges (see Figure 2.1.5). Determine the existence and extent of scour using a grid system and noting the depth of the channel bottom at each grid point.



Figure 2.1.5 Inspection for Scour and Undermining

Note the embankment erosion both upstream and downstream of the bridge, as well as any debris and excessive vegetation. Record their type, size, extent, and location. Note also the high water mark, referencing it to a fixed elevation such as the bottom of the superstructure.

Inspection of Bridge There are several general terms used to describe bridge deficiencies: **Elements**

- Corrosion section loss
- Cracking breaking away without separating in to parts
- Splitting separating in to parts
- Connection slippage relative movement of connected parts
- Overstress deformation due to overload
- Collision damage damage caused when a bridge is struck by vehicles or vessels

Refer to Chapter 6 for a more detailed list and description of types and causes of deterioration for specific materials. As described in Chapter 6, each material is subject to unique deficiencies. Therefore, be familiar with the different inspection methods used with each material.

Timber Inspection

When inspecting timber structures, determine the extent and severity of decay, weathering and wear, being specific about dimensions, depths, and locations. Sound and probe the timber to detect hidden deterioration due to decay, insects, or marine borers.

Note any large cracks, splits, or crushed areas. While collision or overload damage may cause these deficiencies, avoid speculation as to the cause and be factual. Note any fire damage, recording the measurements of the remaining sound material. Document any exposed untreated portions of the wood, indicating

the type, size, and location.

Concrete Inspection

When inspecting concrete structures, note all visible cracks, recording their type, width, length, and location. Also record any rust or efflorescence stains. Concrete scaling can occur on any exposed face of the concrete surface, so record its area, location, depth, and general characteristics. Inspect concrete surfaces for delamination or hollow zones, which are areas of incipient spalling, using a hammer or a chain drag. Carefully document any delamination using sketches showing the location and pertinent dimensions.

Unlike delamination, spalling is readily visible. Document any spalling using sketches or photos, noting the depth of the spalling, the presence of exposed reinforcing steel, and any deterioration or section loss that may be present on the exposed reinforcement.

Metal Inspection: Steel, Iron and Others

When inspecting metal structures, determine the extent and severity of corrosion, carefully measuring the amount of cross section remaining. Note all cracks, recording their length, size, and location. Document all bent or damaged members, noting the type of damage and amount of deflection.

Loose rivets or bolts can be detected by striking them with a hammer while holding a thumb on the opposite end of the rivet or bolt. Movement can be felt if it is loose. In addition, note any missing rivets or bolts.

Note any frozen pins, hangers, or expansion devices. One indication of this is if the hangers or expansion rockers are inclined or rotated in a direction opposite to that expected for the current temperature. In cold weather, rocker bearings lean towards the fixed end of the bridge, while in hot weather, they lean away from the fixed end. A locked bearing is generally caused by heavy rust on the bearing elements.

For the evaluation to be substantiated, document and record all inspection findings. Documentation is referred to as the "condition remarks" on the inspection form or in the inspection report.

Masonry Inspection

The examination of stone masonry and mortar is similar to that of concrete. Carefully inspect the joints for cracks and other forms of mortar deterioration. Inspection techniques are generally the same as for concrete.

Check masonry arches or masonry-faced concrete arches for mortar cracks, vegetation, water seepage through cracks, loose or missing stones or blocks, weathering, and spalled or split blocks and stones.

Fiber Reinforced Polymer Inspection

When inspecting Fiber Reinforced Polymers (FRP), note any blistering, voids and delaminations, discoloration, wrinkling, fiber exposure and any scratches. Document all visible cracks, recording their width, length, and location.

Critical Findings	Critical findings are any structural or safety-related deficiency that requires
_	immediate follow-up inspection or action. When a critical finding is discovered,
	immediately communicate and document the critical finding according to agency
	procedures.

Refer to Topic 4.5 for a detailed description of critical findings and the methods required to address any critical findings discovered.

2.1.6

Preparing the Report Documentation is essential for any type of inspection. Gather enough information to ensure a comprehensive and complete report. Report preparation is a duty, which reflects the effort that the inspector puts in to performing the inspection. Both documentation and preparation are to be comprehensive. The report is a record of both the bridge condition and the inspector's work.

Basic activities in preparing the inspection report include:

- Completion of agency forms
- > Objective written documentation of all inspection findings
- Providing photo references and sketches
- Objective evaluation of the bridge, roadway and waterway components and elements
- Recommendations and cost estimates (refer to Topic 2.1.7 for further details)
- Summary

A sample bridge inspection report can be found in Appendix B of this manual. Follow the procedures of the agency responsible for the bridge.

2.1.7

Identifying Items for Preservation and Follow-up for Critical Findings

Another common duty is to identify work recommendations for bridge preservation and follow-up to critical findings. Recommend work items that promote public safety and maximize useful bridge life. Refer to Topic 4.5 for details on follow-up to critical findings.

Work recommendations are commonly aligned with an agency's bridge preservation program and are included in preservation work plans. These work recommendations are condition driven or cyclical. Examples of preservation activities include: deck or bridge washing, flushing the scuppers and down spouts, lubricating the bearings and painting the structure.

Carefully consider the benefits to be derived from completing the work recommendation and the consequences if the work is not completed. Also, check the previous report recommendations to see what work was recommended and the priority of such items. If work was scheduled to be completed before the next inspection, note if the work was completed and the need for any follow-up work.

The NBIS regulation requires the establishment of a statewide or Federal agency wide procedure to assure that critical finds are addressed in a timely manner. Additionally, the NBIS requires that FHWA be periodically notified of actions taken to resolve or monitor critical findings. The duty of the inspection team is to

follow statewide or Federal agency-wide procedures for the follow-up on critical findings. It is the responsibility of Bridge Owners to implement procedures for addressing critical deficiencies, including:

- Immediate critical deficiency reporting steps
- Emergency notification of police and the public
- Rapid evaluation of the deficiencies
- Rapid implementation of corrective or protective actions
- A tracking system to ensure adequate follow-up
- Provisions for identifying other bridges with similar structural details for follow-up inspections

Critical findings are presented in detail in Topic 4.5.

2.1.8

Types of Bridge
InspectionThe type of inspection may vary over the useful life of a bridge to reflect the
intensity of inspection required at the time of inspection. The seven types of
inspections identified in the AASHTO Manual for Bridge Evaluation are described
below and allow a Bridge Owner to establish appropriate inspection levels
consistent with the inspection frequency and the type of structure and details.

- **Initial (Inventory)** An initial inspection is the first inspection of a bridge as it becomes a part of a bridge file, but the elements of an initial inspection may also apply when there has been a change in configuration of the structure (e.g., widening, lengthening, supplemental bents, etc.) or a change in bridge ownership. The initial inspection is a fully documented investigation and is accompanied by load capacity ratings. The purpose of this inspection is two-fold. First, an initial inspection provides all Structure Inventory and Appraisal (SI&A) data. Second, it provides baseline structural conditions and identification of existing problems.
- **Routine (Periodic)** Routine inspections are regularly scheduled inspections consisting of observations and/or measurements needed to determine the physical and functional condition of the bridge, to identify any changes from "initial" or previously recorded conditions, and to ensure that the structure continues to satisfy present service conditions. Inspection of underwater portions of the substructure is limited to observations during low-flow periods and/or probing for signs of scour and undermining. The areas of the structure to be closely monitored are those determined by previous inspections and/or load rating calculations to be critical to load-carrying capacity. Follow the plan of action for scour critical bridges.

According to the NBIS, inspect each bridge at regular intervals not to exceed 24 months. However, certain bridges require inspection at less than the 24-month interval. Establish criteria to determine inspection frequency and intensity based on such factors as age, traffic characteristics, and known deficiencies. Certain bridges may be inspected at greater than 24-month intervals, not to exceed 48 months, with prior FHWA-approval. This may be appropriate when past inspection findings and analysis justifies the increased inspection interval.

DamageA damage inspection is an unscheduled inspection to assess structural damage
resulting from environmental factors or human actions. The scope of inspection is
sufficient to determine the need for emergency load restrictions or closure of the
bridge to traffic and to assess the level of effort necessary for an effective repair.

In-Depth An in-depth inspection is a close-up, inspection of one or more members above or below the water level to identify any deficiencies not readily detectable using routine inspection procedures. Hands-on inspection may be necessary at some locations. When appropriate or necessary to fully ascertain the existence of or the extent of any deficiencies, nondestructive field tests may need to be performed. The inspection may include a load rating to assess the residual capacity of the member or members, depending on the extent of the deterioration or damage. This type of inspection can be scheduled independently of a routine inspection, though generally at a longer interval, or it may be a follow-up for other inspection types. For small bridges, the in-depth inspection includes all critical members of the structure. For large and complex structures, these inspections may be scheduled separately for defined segments of the bridge or for designated groups of elements, connections, or details.

According to the NBIS, establish criteria to determine the level and frequency of this type of inspection.

Fracture Critical A fracture critical member (FCM) inspection is performed within arm's length of steel members in tension, or with a tension element, who failure would probably cause a portion of or the entire bridge to collapse. The FCM inspection uses visual methods that may be supplemented by nondestructive testing. A very detailed visual hands-on inspection is the primary method of detecting cracks. This may require that critical areas be specially cleaned prior to the inspection and additional lighting and magnification be used. Other nondestructive methods may be used at the discretion of the Bridge Owner. Where the fracture toughness of the steel is not documented, some tests may be necessary to determine the threat of brittle fracture at low temperatures.

According to the NBIS, fracture critical members (FCMs) are to be inspected at regular intervals not to exceed 24 months. However, certain FCMs require inspection at less than 24-month intervals. Establish criteria to determine the inspection level and frequency to which these members are inspected considering such factors as age, traffic characteristics, and known deficiencies.

Underwater An underwater inspection is the inspection of the underwater portion of a bridge substructure and the surrounding channel, which cannot be inspected visually at low water by wading or probing, generally requiring diving or other appropriate procedures. Underwater inspections are an integral part of a total bridge inspection plan. Scour evaluations are conducted for all bridges over water. Determine the severity and extent of scour, immediately communicating and documenting critical findings. Follow the plan of action for scour critical bridges.

Structural damage, scour and erosion due to water movement, drift, streambed load, ice loading, navigation traffic collision, and deleterious effects of water movement or of elements, are typical occurrences that could result in the decision to conduct underwater inspections at shorter intervals.

According to the NBIS, underwater structural elements are inspected at regular intervals not to exceed 60 months. However, certain underwater structural elements require inspection at less than the 60-month intervals. Establish criteria to determine the level and frequency to which these members are inspected considering such factors as construction material, environment, age, scour characteristics, condition rating from past inspections and known deficiencies. Certain underwater structural elements may be inspected at greater than 60-month intervals, not to exceed 72 months, with written FHWA-approval. This may be appropriate when past inspection findings and analysis justifies the increased inspection interval.

Special (Interim) A special inspection is an inspection scheduled at the discretion of the Bridge Owner. It is used to monitor a particular known or suspected deficiency, such as foundation settlement or scour, fatigue damage, or the public's use of a load posted bridge. These inspections are not usually comprehensive enough to meet NBIS requirements for routine inspections.

According to the NBIS, establish criteria to determine the level and frequency of this type of inspection. Guidelines and procedures on what to observe and/or measure are provided, and a timely process to interpret the field results is in place.

This page intentionally left blank.

Table of Contents

Chapter 2 Safety Fundamentals for Bridge Inspectors

2.2 Safety	Fundamentals for Bridge Inspectors 2.2.1
2.2.1	Importance of Bridge Inspection Safety 2.2.1
2.2.2	Safety Responsibilities
2.2.3	Personal Protection2.2.2Proper Inspection Attire2.2.2Inspection Safety Equipment2.2.2Hard Hat2.2.3Reflective Safety Vest2.2.3Safety Goggles2.2.4Gloves2.2.4Life Jacket2.2.5Dust Mask/Respirator2.2.6Safety Harness and Lanyard2.2.7Boats/Skiff2.2.8
2.2.4	Causes of Accidents
2.2.5	Safety Precautions2.2.9General2.2.9Mental Attitude2.2.9General Guidelines2.2.9Working in Teams2.2.10Climbing Safety2.2.10Organization2.2.11Inspection Equipment2.2.12Scaffolding2.2.13Timber Planks2.2.13Catwalks and Travelers2.2.13Rigging2.2.14Confined Spaces Precautions2.2.15Safety Procedures2.2.16Vegetation (poison ivy, sumac)2.2.16Nightwork2.2.17Wading2.2.17Drowning2.2.17

CHAPTER 2: Fundamentals of Bridge Inspection TOPIC 2.2: Safety Fundamentals for Bridge Inspectors

Underwater	
Culverts	2.2.17
Inadequate Ventilation	
Quicksand Conditions at the Outlet	
Working Around Traffic	

Topic 2.2 Safety Fundamentals for Bridge Inspectors

2	2	1
4	. _	.1

Importance of Bridge Inspection Safety	While safety danger membe import bridge	completing the inspection in a timely and efficient manner is important, is also a major concern in the field. Bridge inspection is inherently ous and therefore requires continual watchfulness on the part of each er of the inspection team. Attitude, alertness, and common sense are three ant factors in maintaining safety. To reduce the possibility of accidents, inspectors need to be concerned about safety.
	4	Injury and pain - Accidents can cause pain, suffering, and even death. Careless inspectors can severely injure or even kill themselves or others on the inspection team. Resulting pain and discomfort can hamper the inspector for the rest of their life.
	\mathbf{A}	Family hardship - A worker's family also suffers hardship when an accident occurs. Not only is there loss of income, but there is also the inability to participate in family activities. In the case of major disability, the burden of caring for the injured person falls on family members.
		Equipment damage - The repair or replacement of damaged equipment can be very costly. There is also a cost associated with the loss of time while the equipment is not available for use.
	>	Lost production - The employer loses revenues associated with the employee's work, and also loses time and money spent on safety training and equipment. Training additional inspectors to replace the injured worker contributes to lost production. Lost production also affects the bridge owner in terms of loses in revenue, time and money if a bridge is closed longer than expected after an inspection accident.
	>	Medical expenses - Whether coverage is an employee benefit, personal insurance, or out of pocket, someone has to pay for medical expenses. Ultimately, everyone is impacted by accidents through higher insurance premiums.

Constantly be aware of safety concerns. Spending the effort to be safe pays big dividends in avoided expenses and grief.

2.2.2				
Safety	The e	The employer is responsible for providing a safe working environment, including:		
Responsibilities	\succ	Clear safety regulations and procedures		
	\succ	Safety training		
	\triangleright	Proper tools and equipment		
	The inclue	supervisor is responsible for maintaining a safe working environment, ding:		
	\triangleright	Supervision of established job procedures		
	\succ	Training in application of safety procedures		
	\succ	Training in proper use of equipment		
	\succ	Enforcement of safety regulations		
	Bridg	ge inspectors are ultimately responsible for their own safety. The bridge ctor's responsibilities include:		
	\triangleright	Recognition of physical limitations – Recognize your limitations and communicate them to your supervisor and inspection team members.		
		Knowledge of rules and requirements of job – Verify that you understand a particular task and that you are qualified to perform that task. If a procedure appears to be unsafe, question it and constructively try to develop a safer procedure.		
		Safety of fellow workers – Do not act in a manner that endangers fellow inspectors. Warn co-workers if they are doing something unsafe.		
	>	Reporting an accident – If there is an accident, it is essential to report it to a designated individual in your agency or company within the prescribed time frame, usually within 24 hours. Promptly report any injury in order to assure coverage, if necessary, under workmen's compensation or other insurance.		

2.2.3 Personal Protection

Proper Inspection Attire	It is important to dress properly for the job. Be sure to wear field clothes that are properly sized and appropriate for the climate. For general inspection activities, wear boots with traction lug soles. For climbing of bridge components, wear boots with a steel shank (with non-slip soles without heavy lugs), as well as gloves. Wearing a tool pouch enables the inspector to carry tools and notes with hands free for climbing and other inspection activities.
Inspection Safety Equipment	Safety equipment is designed to prevent injury. Use the equipment correctly in order for it to provide protection. The following are some common pieces of safety equipment:

Hard Hat

A hard hat can prevent serious head injuries in two ways. First, it provides protection against falling objects. The bridge site environment during inspection activities is prone to falling objects. Main concerns are:

- > Deteriorated portions of bridge components dislodged during inspection
- Equipment dropped by coworkers overhead
- Debris discarded by passing motorists

Secondly, a hard hat protects the inspector's head from accidental impact with bridge components. When inspections involve climbing or access equipment, the inspector is frequently dodging various configurations of superstructure elements. These superstructure elements can be sharp edged and are always unyielding. If the inspector makes a mistake in judgement during a maneuver and impacts the structure, a hard hat may prevent serious injury.

It is a good practice to always wear a hard hat (see Figure 2.2.1). Also, if the inspector is free climbing, it is a good practice to wear a chinstrap with the hard hat.



Figure 2.2.1 Inspector Wearing a Hard Hat

Reflective Safety Vest

When performing activities near traffic, the inspector is required to wear a safety vest. Be sure the vest conforms to current OSHA and MUTCD standards. The combination of bright color and reflectivity makes the inspector more visible to passing motorists. Safety is improved when the motorist is aware of the inspector's presence (see Figure 2.2.2).



Figure 2.2.2 Inspector Wearing a Reflective Safety Vest

Safety Goggles

Eye protection is necessary when the inspector is exposed to flying particles. Glasses with shatterproof lenses are not adequate if side protection is not provided. It is also important to note that only single lens glasses be worn when climbing (no bifocals).

Wear eye protection during activities such as:

- ➢ Using a hammer
- ➢ Using a scraper or wire brush
- ➢ Grinding
- Shot or sand blasting
- Power tools

Gloves

Although one may not immediately think of gloves as a piece of safety equipment, they can prove to be an important safety feature. Wearing gloves protect the inspector's hands from harmful effects of deteriorated members (see Figure 2.2.3). In many inspections, structural members have been deteriorated to the point where the edges of the members have become razor sharp. These edges can cause severe cuts and lacerations to the inspector's hands that may become infected.



Figure 2.2.3 Inspector Wearing Safety Goggles and Gloves

Life Jacket

Always wear a life jacket when working over water or in a boat (see Figure 2.2.4). If an accident occurs, good swimmers may drown if burdened with inspection equipment. Also, if knocked unconscious or injured due to a fall, a life jacket keeps the inspector afloat. Also wear a life jacket when wearing hip or chest waders. If an inspector slips or steps in an area that is too deep, their waders can fill with water and drag them under, making swimming impossible.



Figure 2.2.4 Inspector Wearing a Life Jacket

Dust Mask / Respirator

A respirator or dust mask can protect the inspector from harmful airborne contaminants and pollutants (see Figure 2.2.5). Consult agency or OSHA regulations for approved types and appropriate usage.

Conditions requiring a respirator include:

- ➢ Sand blasting
- > Painting
- Exposure to dust from pigeon droppings (exposure to pigeon droppings may result in histoplasmosis, a potentially very serious illness)
- > Work in closed or constricted areas
- Hammering, scraping or wire brushing steel members with lead based paints.



Figure 2.2.5 Inspector Wearing a Respirator

Safety Harness and Lanyard

The safety harness and lanyard is the inspector's lifeline in the event of a fall (see Figure 2.2.6). Use this equipment as required by conditions. Make sure you satisfy agency and OSHA requirements.

For example, some agencies require a safety harness be worn in the following situations:

- ➤ At heights over 20 feet
- > Above water
- Above traffic



Figure 2.2.6 Inspector with Safety Harness with a Lanyard

To reduce the possibility of injury, the maximum lanyard length limits a fall to 6 feet per OSHA regulations. Further protection can be achieved using a shock absorber between the lanyard and the safety harness. The shock absorber reduces g-forces through the controlled extension of nylon webbing, which is pre-folded and sewn together. Two lanyards are required with one lanyard being tied off to a solid structural member or to a safety line rigged always for this purpose. Use the second lanyard to allow safe movement around obstacles connecting the second lanyard before disconnecting the first lanyard in order to safely move along the structure.

Do not tie off to scaffolding or its supporting cable. One of the reasons for tying off is to limit your fall in case the rigging or scaffold fails. When working from a under bridge inspection vehicle or bucket truck, tie off to the structure if possible. Exercise extreme caution not to allow the equipment to be moved out from under

someone tied to the bridge. If the machine is being moved frequently, it is best to tie off to the bucket or boom.

Boats/Skiff

There must be one rescue person present and specifically assigned to respond to water emergencies at all times when the inspection over water is active. Whenever possible, a manned boat/skiff should be in the water. Whenever possible, a manned boat/skiff should be in the water. In the event of an accident in which someone was to fall in to the water, the boat can rescue them quickly. This is especially important if the individual has been rendered unconscious. In addition, it can also be used to retrieve any equipment that may have been accidentally dropped by an inspector. In situations where the use of a boat/skiff is not practical, a rescue person should be stationed on the river bank with a life ring.

2.2.4 Causes of Accidents

General Causes	Accidents are usually caused by human error or equipment failure. Part of safety awareness is acknowledging this and planning ahead to minimize the effects of those errors or failures.		
	Accide improp minimi judgme	nts caused by equipment failure can often be traced to inadequate or er maintenance. Inspection, maintenance, and update of equipment can ze failures. Accidents caused by people are usually caused by an error in ent, thoughtlessness, or trying to take shortcuts.	
Specific Causes	Specifi	c causes of accidents include the following:	
	\blacktriangleright	Improper attitude – distraction, carelessness, worries over personal matters.	
		Personal limitations – lack of knowledge or skill, exceeding physical capabilities.	
		Physical impairment – previous injury, illness, side effect of medication, alcohol or drugs.	
		Boredom – falling into an inattentive state while performing repetitive, routine tasks.	
	\succ	Thoughtlessness - lack of safety awareness and not recognizing hazards.	
	\succ	Shortcuts - sacrificing safety for time.	
		Faulty equipment – damaged ladder rungs, worn rope, frayed cables or access equipment not inspected regularly.	
		Inappropriate or loose fitting clothing.	

2.2.5				
Safety Precautions	Safet Clime Wate	Safety precautions can be divided in to several categories: General Precautions, Climbing Safety, Confined Spaces, Vegetation, Night Work, Working Around Water, and Culverts.		
General	Ment	al Attitude		
	The inspector has to be mentally prepared to do a climbing inspection. A good safety attitude is of foremost importance. Address the following three precautions:			
		Avoid emotional distress – Do not climb when emotionally upset. The inspector who climbs needs to have complete control; otherwise the chances of falling increase.		
		Awareness of surroundings – Always be aware of dangers associated with inspection location when climbing. Do not become as engrossed in the job as to step into mid-air.		
		Realize limitations – An inspector is to be confident the job can be performed safely. If there is a feature that cannot safely be inspected with the equipment available, do not inspect it. Highlight this fact in the notes so that appropriate equipment can be scheduled if necessary. Do not hide the fact that a particular bridge member was not inspected.		
	General Guidelines			
	Some general guidelines for safe inspections are as follows:			
	\blacktriangleright	Keeping well rested and alert – Working conditions encountered during an inspection are varied and can change rapidly requiring the inspector be fit and attentive.		
	\mathbf{A}	Maintaining proper mental and physical condition – Inspection tasks require a multitude of motor skills. To perform at acceptable levels, the inspector is to be physically fit and free from mental distractions.		
		Using proper tools – Do not try to use tools and equipment not suited for the job.		
		Keeping work areas neat and uncluttered – Tools and equipment scattered carelessly about the work area present hazards that can result in injury.		
		Establishing systematic methods – Establish methods early in the job and utilize them so everyone knows what to expect of one another.		
		Follow safety rules and regulations – Adhere to the safety rules and regulations established by the OSHA, the agency, and your employer.		
	\blacktriangleright	Use common sense and good judgment – Do not engage in horseplay, and do not take short cuts or foolish chances.		
	\triangleright	Do not use of intoxicants or drugs – Intoxicants impair judgment, reflexes,		

Medication – Prescription and over-the-counter medications can cause drowsiness or other unwanted and potentially dangerous side effects.

and coordination.

- Electricity This is a potential killer. Assume cables and wires to be hot (live) even if they appear to be only telephone cables. The conditions encountered on many bridges are conducive to electric shock. These conditions include steel members, humidity, perspiration, and damp clothing. Identify transmission lines on a structure prior to the inspection. Shut down power lines. In rural areas, avoid electric fences since they can be a hazard. Be aware that fiberglass posts eliminate the need for the distinctive porcelain insulation, which once identified electric fences.
- Inspection over water A safety boat is to be provided when working over water. Be sure the boat is equipped with a life ring and radio communication with the inspection crew.
- ➤ Waders Use caution when wearing waders. If the inspector falls into a scour hole, the waders can fill with water, making swimming impossible.
- ▶ Inspection over traffic It is best to avoid working above traffic. If it cannot be avoided, tie off equipment, such as hand tools and clip boards.
- Entering dark areas Use a flashlight to illuminate dark areas prior to entering as a precaution against falls, snakebites, and stinging insects.
- Vagrant people Exercise caution when approaching a bridge where homeless people are present. Explain to them an inspection of the bridge is taking place, and the inspection team leaves the site as soon as possible. Leave the bridge site immediately if there are any illegal activities or perceived danger.

Working in Teams

Work in pairs. Do not take any action without someone else there to help in case of an accident. Make sure someone else knows where you are. If someone seems to be missing, locate that person immediately.

First Aid Training is recommended for bridge inspectors and is available through organizations such as OSHA or the American Red Cross.

If an inspector is injured during an inspection, it is important to know First Aid and/or cardiopulmonary resuscitation (CPR). The American Red Cross offers training for First Aid, CPR and AED (automatic external defibrillator). Local fire departments and the American Heart Association (AHA) can also provide training for CPR.

- Climbing Safety There are two primary areas of preparation necessary for a safe climbing inspection:
 - ➢ Organization
 - Inspection Equipment



Figure 2.2.7 Inspection Involving Extensive Climbing

Organization

Organization of the Inspection - A good inspection procedure incorporates a climbing strategy that minimizes climbing time. For example, beginning the day with an inspection of a truss span from one bent and finishing at the next bent by lunch time eliminates unproductive climbing across the span.

The inspection procedure needs to have an inspection plan so the inspection team knows where to go, what to do, and what tools are needed to perform the inspection. An organized inspection reduces the chance of the inspectors falling or getting stuck in a position in which they are unable to get down.

Weather conditions are a primary consideration when organizing a climbing inspection. Moderate temperatures and a sunny day are desirable.

Rain conditions warrant postponement of steel bridge inspections, as wet steel is extremely slippery.

After a rainy day, be sure that your boots are free of mud, and use extreme caution in areas where debris accumulation may cause a slippery surface.

Inspection Equipment

The inspection team needs to be well equipped to properly complete their inspection.

Check personal attire for suitability to the job:

Clothing – proper for climbing activities and temperature.

- Jewelry Avoid wearing rings, bracelets, and necklaces. In an accident, jewelry can become snagged and cause additional injury.
- Eyeglasses wear only single lens glasses; do not wear bifocals because split vision impairs ability to climb safely.

Check inspection equipment for proper use and condition.

Ladders

Accidents involving ladders are the most common type of inspection-related accident. Refer to and follow OSHA for rules applicable to stairways and ladders.

In order to use a ladder properly, consider the following:

- Proper ladder length for the job.
- \blacktriangleright 4 to 1 tilt with blocked and secured bottom (see Figure 2.2.8).
- An assistant for ladders over 25 feet, and making sure the top is tied off.
- > Inspecting the ladder, prior to use, for cracked or defective rungs and rails.
- Correct climbing technique using both hands, facing the ladder, and keeping the inspector's center of gravity or belt buckle over the rungs.
- Using a hand line to lift equipment or tools.



Figure 2.2.8 Proper Use of Ladder

Scaffolding

Refer to and follow OSHA for rules applicable to scaffolding. Check scaffolding for the height and load capacity necessary to support the inspection team.

Load tests can be performed on the ground with planned equipment and personnel. Perform a daily inspection for cracks, loose connections, and buckled or weak areas prior to use.

Timber Planks

Never use single planks. Use two or more planks securely cleated together. Securely attach plank ends to their supports. Inspect planks for knots, splits, cracks, and deterioration prior to use.

Inspection Vehicles

Use of platform trucks, bucket trucks, and underbridge inspection vehicles may be necessary to access elements during an inspection (see Figure 2.2.9). Confirm that they are in safe operating condition. Only use such equipment when placed on a firm surface at a slope not exceeding the manufacturer's recommendations. Use extreme caution when operating near traffic.



Figure 2.2.9 Bucket Truck

Catwalks and Travelers

Permanent inspection access devices are ideal. However, be on guard for misalignment and deterioration of elements, such as flooring, hand-hold rods, and cables (see Figure 2.2.10).



Figure 2.2.10 Inspection Catwalk

Rigging

Be familiar with proper rigging techniques. The support cables need to be at least one-half inch in diameter. The working platform or "stage" need to be at least 20 inches wide. Use a line or tie-off cable separate from the primary rigging.

Use common sense with regard to rigging. Do not blindly trust the people arranging the rigging. Mistakes by riggers can cause life threatening accidents. If a method is unsafe or doubtful, question it and get it changed if necessary. Do not rely on ropes or planks left on the bridge by prior work. They may be rotted or not properly attached.



Figure 2.2.11 Inspection Rigging

Confined Spaces Precautions

Safety Concerns

Inspection of box girder bridges, steel box pier caps, steel arch rings, arch ties, cellular concrete structures, and long culverts is often categorized as confined spaces. Confined space entry is regulated by Occupational Safety and Health Administration (OSHA) and requires proper training, equipment, and permitting.

There are four major concerns when inspecting a confined space:

- Lack of oxygen an oxygen content above 19% is needed for the inspectors to remain conscious
- Toxic gases generally produced by work processes such as painting, burning, and welding or by operation of internal combustion engines
- Explosive gases natural gas, methane, or gasoline vapors may be present naturally or due to leaks
- Lack of light many confined spaces are totally dark (inspector cannot see any potential hazards such as depressions, drop-offs, or dangerous animals)

Safety Procedures

When inspecting a confined area, use the safety standards prescribed by OSHA and any additional agency or employer requirements. The following is a general description of the basic requirements. Refer to OSHA for specifics.

Pre-entry air tests:

- > Test for oxygen with an approved oxygen testing device
- Test for other gases, such as carbon monoxide, hydrogen sulfide, methane, natural gas, and combustible vapors

Mechanical ventilation:

- Pre-entry Check oxygen and gas levels and verify acceptability for a minimum prescribed time prior to entry.
- During occupancy Regardless of activity, use continuous ventilation. Test for oxygen and other gases at prescribed intervals during occupancy.

Basic safety procedures:

- Avoid use of flammable liquids in the confined area.
- Position inspection vehicles away from the area entrance to avoid carbon monoxide fumes.
- Perform operations that produce toxic gases "down-wind" of the operator and the inspection team.
- Position gasoline powered generators "down-wind" of operations.
- Carry approved rescue air-breathing apparatus.
- ➤ Use adequate lighting with an appropriate backup system and lifelines when entering dark areas, such as box girders and culverts.
- Perform the inspection in pairs, with a third inspector remaining outside of dark or confined areas with means to communicate with inspectors.
- **Vegetation** Be aware of any vegetation located around any substructures. Poison ivy, oak and sumac are examples of vegetation which can cause skin irritations if touched by someone. Also, it is important to be aware of any tall vegetation which could hide holes in the ground and lead to possible injury if not found. Tall vegetation can also hide other tripping hazards.
- Night workWhen working at night, it is important to be properly dressed. This is necessary so
the inspectors can be more visible by passing motorists. It can be accomplished by
wearing a safety vest which has both bright colors and reflectivity. The use of
proper temporary traffic control also helps motorists be aware that there are
workers ahead.

Working Around Water Wading

When wading in water, it is important to be aware of any scour holes and be careful not slip or fall on objects in the water. If an inspector slips or steps into a scour hole, their waders can fill with water and drag them under, making swimming impossible. It is also important to wear a life vest while wading to help prevent the inspector from being pulled down if the waders were to fill up with water. It is beneficial for the inspector to carry and use probing rod to locate scour holes and soft stream bed material. Be mindful of potentially dangerous aquatic life.

Drowning

Extensive streambed scour may result in channel depressions. During periods of low flow the depth of water in these holes may be significantly greater than the remainder of the streambed. This could give the inspector the impression that wading is safe. It is advisable that the inspector use a probing rod to check water depth wherever he/she plans to walk.

Storms may generate high flows in culverts very quickly. This creates a dangerous situation for the inspectors. It is not uncommon for culverts to carry peak flow long before a storm reaches the culvert site. Be cautious whenever storms appear imminent.

Underwater

When performing an underwater inspection, particularly in low visibility and/or high current situations, use extreme care and be sure to watch for drift and debris at any height in the water. See Topic 13.3.2, for additional safety concerns.

Culverts There are several hazards that can be encountered when performing a culvert inspection. Being aware of these situations and exercising proper precautions protect the inspector from these dangerous and potentially life threatening hazards. The following are some of the hazardous conditions an inspector may encounter.

- Inadequate Ventilation
- > Drowning
- Quick Sand Conditions at the Outlet
- Potentially dangerous wildlife

Inadequate Ventilation

Culverts with inadequate ventilation can develop low oxygen levels or high concentrations of toxic and/or explosive gases. This is a big concern when one culvert end may be blocked or inspection is being performed on a long culvert.

If air quality is suspect, perform tests to determine the concentration of gases. Testing devices may be as simple as badges worn by inspectors that change colors when in the presence of a particular gas. Devices may also be sophisticated instruments that measure the concentration of several gases. Observe confined space entry requirements when inspecting a long culvert or any culvert with restricted ventilation.

Quicksand Conditions at the Outlet

Quicksand conditions can occur in sandy streambeds, especially at the outlet end of the culvert. Be aware of these conditions and proceed with caution in geographical areas known to have these problems.

Working Around Traffic Do not obstruct traffic during bad weather. Avoid the inspection of the top of concrete decks during or just after it rains (see Figure 2.2.12).



Figure 2.2.12 Inclement Weather Causing Slippery Bridge Members and Poor Visibility for Motorists

Table of Contents

Chapter 2 Safety Fundamentals for Bridge Inspectors

2.3	Temporary Traffic Control	
	2.3.1	Introduction
	2.3.2	Philosophy and Fundamental Principles2.3.1Inform the Motorists2.3.2Control the Motorists2.3.2Provide a Clearly Marked Path2.3.2
	2.3.3	Inspector Safety Practices2.3.3Work Zone2.3.3Vehicles and Equipment2.3.4Workers2.3.4
	2.3.4	Principles Temporary Traffic Control Devices
	2.3.5	Types of Temporary Traffic Control Devices2.3.6Signs2.3.6Channelizing Devices2.3.8Lighting Devices2.3.12Flaggers2.3.13One-lane, Two-way Traffic Control2.3.18Shadow Vehicles2.3.18Police Assistance2.3.19Specialized Traffic Crews2.3.19
	2.3.6	Public Safety2.3.19Training2.3.19Responsibility2.3.20

This page intentionally left blank.
Topic 2.3 Temporary Traffic Control

2.3.1 Introduction

Bridge inspection usually only requires traffic control procedures for a relatively short term closure Long term closures for construction activity which use concrete barriers are not included in this topic.



Figure 2.3.1 Temporary Traffic Control Operation

Bridge inspection, like construction and maintenance activities on bridges, often presents motorists with unexpected and unusual situations. Most state agencies have adopted the *Federal Manual on Uniform Traffic Control Devices for Streets and Highways (MUTCD)*. Some states and local jurisdictions, however, issue their own standard manuals or drawings.

When working in an area exposed to traffic, check and follow the existing agency standards. These standards prescribe the minimum procedures for a number of typical applications and the proper use of standard temporary traffic control devices such as cones, signs, and flashing arrow-boards (see Figure 2.3.1). Sometimes after initial installation, temporary traffic control may need revised to provide adequate protection to motorists, pedestrians or inspectors.

2.3.2				
Philosophy and	Temporary traffic control devices used on street and highway construction or			
Fundamental	indamental maintenance work need to conform to the applicable standards of the <i>MUTCL</i>			
Principles	the agency.			
	Minimize inspection time to reduce exposure to potential hazards without compromising the thoroughness of the inspection. Principles and procedures which have been shown to enhance the safety of motorists, pedestrians, and bridge inspectors in the vicinity of work areas include the following:			

Inform the Motorists Traffic safety in work zones is an integral and high priority element of every inspection project, from the planning stage to performance of the inspection. Keep in mind the safety of the motorist, pedestrian, and worker.

The basic safety principles governing the design of temporary traffic control for roadways and roadsides, govern the design of inspection sites. The goal is to route traffic through such areas with geometrics and temporary traffic control devices comparable to those for normal highway situations. Clearly communicate to the driver the notice of work site locations and guidance through these sites.

A temporary traffic control plan, in detail appropriate to the complexity of the work project, is prepared and understood by the responsible parties before the site is occupied. The official trained in safe traffic control practices approves any changes in the temporary traffic control plan.

Control The Motorists Inhibit traffic movement as little as practical. Design temporary traffic control in work sites on the assumption that motorists only reduce their speeds if they clearly perceive a need to do so. Avoid reducing the speed zoning as much as practical.

The objective is a traffic control plan that uses a variety of temporary traffic control measures and devices in whatever combination necessary to assure smooth, safe vehicular movement past the work area and at the same time provide safety for the equipment and the workers on the job. Avoid frequent and abrupt changes in geometrics, such as lane narrowing, dropped lanes, or main roadway transitions that require rapid maneuvers.

Make provisions for the safe operation of work vehicles, particularly on high speed, high volume roadways. This includes the use of roof mounted flashing lights or flashers when entering or leaving the work zone. This also includes considering the number of lanes that can be closed at one time for an operation. While it might be most cost efficient to inspect the entire floor system from left to right, temporary traffic control may dictate working partial width, a few stringers at a time.

Provide a ClearlyA good traffic control plan provides safe and efficient movement of motorists and
pedestrians and the protection of bridge inspectors at work areas.

Provide adequate warning, delineation, and channelization to assure the motorist positive guidance in advance of and through the work area. Use proper signing and other devices which are effective under varying conditions of light and weather.

The maintenance of roadside safety requires constant attention during the life of the work because of the potential increase in hazards. Remove temporary traffic control devices immediately when no longer needed.

To accommodate run-off-the-road incidents, disabled vehicles or other emergency situations, it is desirable to provide an unencumbered roadside recovery area that is as wide as practical.

Accomplish the channelization of traffic by the use of cones, barricades, and other lightweight devices which yield when hit by errant vehicles.

Store equipment and materials in such a manner as not to be vulnerable to run-offthe-road vehicle impact, whenever practical. Also, provide adequate attenuation devices when safe storage is not available.

2.3.3

Inspector Safety Practices

Work Zone

Traffic represents as great, or even greater, threat to the inspector's safety than climbing high bridges. The work zone is intended to be a safe haven from traffic so the inspectors can concentrate on doing their jobs.

As such, the work zone needs to be clearly marked so as to guide the motorist around it and, insofar as possible, prevent errant vehicles from entering (see Figure 2.3.2). To minimize traffic disruption, the work zone needs to be as compact as possible, but wide enough and long enough to permit access to the area to be inspected and allow for safe movement of workers and equipment. The end of the work zone is to be clearly signed as a courtesy to the motorist.



Figure 2.3.2 Work Zone

Vehicles and Equipment Inspection vehicles and equipment need to be made visible to the motorists with flashing marker lights or arrow boards as appropriate (see Figure 2.3.3).

Use roof mounted flashing lights or flashers on vehicles entering and exiting the work zone to distinguish them from other motorists' vehicles. Also, vehicles are to use extreme caution when moving in and out of the work zone. Allow motorists ample time to react to the vehicle's movements.



Figure 2.3.3 Inspection Vehicles with Flashing Light

Individuals in a work zone are to wear approved safety vests and hard hats for visibility and identification. They also help make the inspector look "official" to the public. Also, it is important for the inspector to stay within the work zone for their own safety.

Workers

2.3.4			
Principles of Temporary Traffic Control Devices	Each b to that locatio of tem tempor	ridge in location n is dep porary t ary traff	spection project is different and has traffic concerns that are unique . Selection of the proper temporary traffic control devices for each endent upon many factors. Though there are several different types raffic control devices, there are some basic principles for efficient ic control devices:
	1.	Tempo Device	prary traffic control devices are to be visible and attention getting. as in good condition are preferred.
		\succ	Bright colors make devices easier for motorists to see. Standard colors are orange and white (<i>MUTCD</i>).
		>	Signs that are legible and color distinguishable at night as well as during the day. Nighttime sign visibility is provided through retroreflectivity, which is accomplished by spherical glass beads or prismatic reflectors in the sign material, or illumination.
		\succ	Properly sized for the roadway so they can be seen by the motorists.
	2.	Tempo	prary traffic control devices are to give clear direction.
	3.	Tempo (<i>MUT</i>) class o gives t	brary traffic control devices are to command respect and be official <i>CD</i>). These devices need to look professional and be geared to the f highway, speeds and traffic involved. Haphazard traffic control he public a bad perception of the rest of the project as well.
		State a workin standar of typ devices	gencies have been mandated to adopt the Federal <i>MUTCD</i> . When ig in an area exposed to traffic, check and follow the agency rds. These standards prescribe the minimum methods for a number ical applications and the proper use of standard traffic control s.
	4.	Tempo proper	prary traffic control devices are to elicit the proper response at the time.
			The decision process includes the classical chain of sensing, perceiving, analyzing, deciding, and responding.
		\blacktriangleright	The average perception-reaction time of a driver is 2.5 seconds. At 60 mph, the 2.5 seconds translates to 220 feet. Additional time and distance is required for a specific action taken such as "hitting the brakes".
		\blacktriangleright	Temporary traffic control accommodates a wide range of vehicles (from small compact cars to large combination tractor-trailers) and

Temporary traffic control accommodates a wide range of vehicles (from small compact cars to large combination tractor-trailers) and driver skills, which may be impaired by alcohol, drugs, drowsiness, or use of cell phones.

Advance warning is essential to get the right response from drivers. The *MUTCD* provides guidance on the positioning of advance warning signs

for specific traffic control applications.

These basic principles for temporary traffic control devices have been factored into the various agencies' procedures for work area traffic control. These procedures represent efforts by trained people. Do not change traffic patterns without consulting the *MUTCD*, agency standards or traffic control personnel.

2.3.5				
Types of	Types	of temporary traffic control signs include the following:		
Temporary Traffic	Types	Types of temporary dame control signs include the following.		
Control Devices		Regulatory – Inform motorists of traffic laws or regulations and indicate the applicability of legal requirements that are not apparent. These signs		
Signs	are authorized by the public agency or official having jun Examples include "Speed Limit", "DO NOT PASS", which ma special authority (see Figure 2.3.4).	are authorized by the public agency or official having jurisdiction. Examples include "Speed Limit", "DO NOT PASS", which may require special authority (see Figure 2.3.4).		
		Warning – Notify road users of specific situations or conditions on or adjacent to a roadway that might not be apparent. They may be used by themselves or in combination with other advance warning signs. Examples include "Bridge Inspection", "Work Area Ahead", and "Slow" messages (see Figure 2.3.5).		
		Guide Signs - Directional and destination signs that provide motorists with information to help them through a temporary traffic control zone. They are not used for bridge inspection traffic control unless a detour is established (see Figure 2.3.6).		
	•	Arrow boards – Used to advise approaching motorists of a lane closure along major multi-lane roadways in situations involving heavy traffic volumes, and/or limited sight distances, or at other location and under other conditions where road users are less likely to expect such lane closures. Use them in combination with the appropriate signing, channelization devices and other temporary traffic control devices (see Figure 2.3.7)		
	>	Changeable message signs – Provide motorists with the notice of unexpected situations. They may present the motorist with complex messages, important information, and real time conditions.(see Figure 2.3.8)		



Figure 2.3.4 Regulatory Sign



Figure 2.3.5 Warning Sign



Figure 2.3.6 Examples of Guide Signs



Figure 2.3.7 Arrow Board



Figure 2.3.8 Changeable Message Sign

Channelizing Devices The functions of channelizing devices are to warn and alert drivers of hazards created by construction or maintenance activities in or near the traveled way and to guide and direct drivers safely past the hazards.

Devices used for channelization provide a smooth and gradual transition in moving traffic from one lane to another, onto a bypass or detour, or in reducing the width of the traveled way. They need to be constructed so as not to inflict any undue damage to a vehicle that inadvertently strikes them.

Channelizing devices are elements in a total system of traffic control devices for use in highway construction and maintenance operations. These elements are preceded by a subsystem of warning devices that are adequate in size, number, and placement for the type of highway on which the work is to take place. Typical channelizing devices include the following:

- Cones Used to channelize motorists, divide opposing traffic lanes, divide lanes when two or more lanes are kept open in the same direction, and delineate short duration maintenance and utility work including bridge inspections. Predominately orange and made of material that can be struck without causing damage to the impacting vehicle and are primarily used during the day. Consult the appropriate governing agency and *MUTCD* to determine the specific requirements for cones, such as size and features, which depend on the application. (see Figure 2.3.9) Drums Used for road user warning or channelization and are constructed from lightweight, deformable materials. They provide the motorist a highly visible and respectable warning of upcoming conditions. For the bridge inspector, drums are portable enough to be shifted place to place within a work zone to accommodate changing conditions (see Figure 2.3.10).
- ➤ Tubular markers Predominately orange and made of a material that can be struck without causing damage to an impacting vehicle. Consult the appropriate governing agency and *MUTCD* to determine the specific requirements for tubular markers, which depends on the application. These devices are not as common for bridge inspection as cones (see Figure 2.3.11).
- Vertical panels May be used to channelize vehicular traffic, divide opposing lanes, or replace portable lightweight barricades. The diagonal orange and white stripes pointing downward indicate the direction motorists are to pass (see Figure 2.3.12).
- Temporary traffic barrier Not considered temporary traffic barriers by themselves. When placed in a position identical to a line of channelizing devices and marked and/or equipped with appropriate channelization features to provide guidance, they serve as traffic control devices. Not only do they serve to direct motorists, but they also protect the workers. These are seldom applicable to bridge inspection due to the short duration of the work (see Figure 2.3.13).



Figure 2.3.9 Cones



Figure 2.3.10 Drums



Figure 2.3.11 Tubular Marker



Figure 2.3.12 Vertical Panel



Figure 2.3.13 Temporary Traffic Barriers

Lighting Devices

Another type of control device is lighting. Lighting devices are used to supplement retroreflectorized signs, barriers, and channelizing devices. Examples of lighting include the following:

- Warning lights Attached to signs or other devices to attract attention or for night visibility. Flashers are commonly placed on maintenance and inspection vehicles, as well as drums, vertical posts, and other channelization devices (see Figure 2.3.14).
- Floodlights Inspection, utility, maintenance, or construction activities are sometimes conducted during nighttime periods when vehicular traffic volumes are lower. Bridge inspections may be conducted during these hours on high volume roadways to avoid additional congestion from daytime traffic. During these periods, floodlights used to illuminate the work area, equipment crossings, and other areas.



Figure 2.3.14 Warning Lights

Flaggers

A number of hand signalizing devices, such as STOP/SLOW paddles, flashing lights, flashlights, and red flags, are used to control traffic through work zones. The sign paddle bearing the clear messages "STOP" or "SLOW" provides motorists with more positive guidance than flags and is generally the primary hand signaling device. If permitted by the agency, limit flag use to emergency situations and at spot locations that can best be controlled by a single flagger.

Since flaggers are responsible for human safety and make the greatest number of public contacts of any inspection personnel, it is important that qualified personnel be selected. The following are qualifications for a flagger:

- Ability to receive and communicate specific instructions clearly, firmly, and courteously
- Ability to move and maneuver quickly in order to avoid danger from errant vehicles
- Ability to control signaling devices (such as paddles and flags) in order to provide clear and positive guidance to drivers approaching a TTC zone in frequently changing situations
- Ability to understand and apply safe traffic control practices, sometimes in stressful or emergency situations
- Ability to recognize dangerous traffic situations and warn workers in sufficient time to avoid injury

For daytime and nighttime activity, flaggers shall wear high-visibility safety apparel that meets Performance Class 2 or 3 requirements of the ANSI/ISEA 107-2004 publication titled *American National Standard for High-Visibility Apparel and Headwear*. The apparel background (outer) material shall be fluorescent orange-red, fluorescent yellow-green, or a combination of the two as defined in the ANSI standard. The retroreflective material shall be orange, yellow, white, silver,

yellow-green, or a fluorescent version of these colors, and shall be visible at a minimum distance of 1,000 feet. The retroreflective safety apparel shall be designed to clearly identify the wearer as a person.

For nighttime activity, high-visibility safety apparel that meets the Performance Class 3 requirements of the ANSI/ISEA 107-2004 publication should be considered for flagger wear.

Flaggers are provided at work sites to stop traffic intermittently as necessitated by work progress. They also maintain continuous traffic past a work site at reduced speeds to help protect the work crew. For both of these functions, the flagger is always clearly visible to approaching traffic for a distance sufficient to permit proper response by the motorist to the flagging instructions and to permit traffic to reduce speed before entering the work site. In positioning flaggers, consideration is given to maintaining color contrast between the work area background and the flagger's protective garments.



Figure 2.3.15 Use of Hand Signaling Devices by Flagger (from Manual on Uniform Traffic Control Devices (MUTCD))

Use the following methods of signaling with sign paddles (see Figure 2.3.15):

➤ To stop traffic, the flagger faces the motorists and aims the STOP paddle face toward the traffic in a stationary position with the arm extended horizontally away from the body. The free arm is held with the palm of the hand above shoulder level toward approaching traffic.

- ➤ To direct traffic to proceed, the flagger faces the motorists with the SLOW paddle face aimed toward the traffic in a stationary position with the arm extended horizontally away from the body. The flagger motions with the free hand for traffic to proceed.
- ➤ To alert or slow traffic, the flagger faces the motorists with the SLOW paddle face aimed toward the motorists in a stationary position with the arm extended horizontally away from the body.

Use the following methods of signaling with a flag (see Figure 2.3.15):

- ➤ To stop traffic, the flagger faces the motorists and extends the flag staff horizontally across the traffic lane in a stationary position so that the full area of the flag is visibly hanging below the staff. The free arm may be held with the palm of the hand above shoulder level toward approaching traffic.
- > To direct stopped traffic to proceed, the flagger faces the motorists with the flag and arm lowered from the view of the drivers, and motions with the free hand for road users to proceed. Flags are not to be used to signal road users to proceed.
- > To alert or slow traffic, the flagger faces the motorists and slowly waves the flag in a sweeping motion of the extended arm from shoulder level to straight down without raising the arm above a horizontal position. The flagger keeps the free hand down.

For flagging traffic at night, lights approved by the appropriate highway authority or reflectorized sign paddles or reflectorized flags are used.

Whenever practicable, the flagger advises the motorist of the reason for the delay and the approximate period that traffic is halted. Flaggers and operators of machinery or trucks are made to understand that every reasonable effort is to be made to allow the driving public the right-of-way and prevent excessive delays.

Locate flagger stations far enough in advance of the work site so that approaching traffic have sufficient distance to reduce speed before entering the project. This distance is related to the approach speed and physical conditions at the site (see Figure 2.3.16). In urban areas, where speeds are low and streets are closely spaced, the distance is decreased.

CHAPTER 2: Fundamentals of Bridge Inspection TOPIC 2.3: Temporary Traffic Control

Speed*	Distance
20 mph	115 feet
25 mph	155 feet
30 mph	200 feet
35 mph	250 feet
40 mph	305 feet
45 mph	360 feet
50 mph	425 feet
55 mph	495 feet
60 mph	570 feet
65 mph	645 feet
70 mph	730 feet
75 mph	820 feet

* Posted speed, off-peak 85th-percentile speed prior to work starting, or the anticipated operating speed

Figure 2.3.16 Stopping Sight Distance as a Function of Speed (from Manual on Uniform Traffic Control Devices (MUTCD))

The flaggers stand either on the shoulder adjacent to the traffic being controlled or in the barricaded lane (see Figure 2.3.17). At a spot obstruction, a position may have to be taken on the shoulder opposite the barricaded section to operate effectively. Under no circumstances is a flagger to stand in the lane being used by moving traffic. The flagger always is clearly visible to approaching traffic. For this reason, the flagger has to stand alone, never permitting a group of workers to congregate around the flagger station. The flagger is stationed sufficiently in advance of the work force to warn them of approaching danger, such as out-ofcontrol vehicles.



Figure 2.3.17 Flagger with Stop/Slow Paddle

Adequately protect flagger stations and precede them by proper advance warning signs. At night, adequately illuminate flagger stations.

At short lane closures where adequate sight distance is available for the safe handling of traffic, the use of one flagger may be sufficient.

One-lane, Two-way Traffic Control Where traffic in both directions use a single lane for a limited distance, make provisions for alternate one-way movement to pass traffic through the constricted work zone. At a spot obstruction, such as a short bridge, the movement may be self-regulating. However, where the one-lane section is of any length, there needs to be some means of coordinating movements at each end so that vehicles are not simultaneously moving in opposite directions in the work zone and so that delays are not excessive at either end. Chose control points at each end of the route so as to permit easy passing of opposing lines of vehicles.

Alternate one-lane, two-way temporary traffic control may be facilitated by the following means:

- Flagger control
- ➢ Flag transfer
- Pilot car
- Temporary traffic signals
- Stop or yield control

Flagger control is usually used for bridge inspection, where the one-lane section is short enough so that each end is visible from the other end. Traffic may be controlled by means of a flagger at each end of the section. Designate one of the two as the chief flagger to coordinate movement. They are able to communicate with each other verbally or by means of signals. These signals are not such as to be mistaken for flagging signals.

Where the end of a one-lane, two-way section is not visible from the other end, the flaggers may maintain contact by means of radio or cell telephones. So that a flagger may know when to allow traffic to proceed into the section, the last vehicle from the opposite direction can be identified by description or license.

- Shadow Vehicles Shadow Vehicles with truck mounted attenuators (TMAs) are used to prevent vehicles from entering the work zone if the motorist drifts into the lane closure. Each agency has its own specific requirements, but a shadow vehicle is generally employed any time a shoulder or travel lane is occupied by workers or equipment. Shadow vehicles are equipped with appropriate lights and warning signs which may be used for stationary operations for additional protection of occupants and vehicles within the work zone.
 - The requirements for the truck itself vary, but high visibility with flashing lights, a striped panel, or an arrow board on the rear of a vehicle of a specified minimum weight is generally required.
 - Some agencies recommend the use of truck or trailer mounted attenuators (see Figure 2.3.18). This protects the motorist, as well as the inspectors.



Figure 2.3.18 Shadow Vehicle with Attenuator

Police Assistance On some inspection projects, police assistance may be helpful and even required. The presence of a patrol car aids in slowing and controlling the motorists. At a signalized intersection near a job site, a police officer may be required to ensure traffic flows properly and smoothly.

Specialized Traffic Crews Some states have specialized traffic crews for high traffic roads. They are used due to their specialized training, allowing for a safer work environment.

2	.3	.6	
			_

Public Safety	Since the fundamental goal of bridge inspection is to enhance public safety, it makes little sense to endanger that same public by inadequate traffic control
	measures. Temporary traffic control does take time, money, and effort. It is, however, a necessary part of the business of bridge inspection.

In the broadest sense, the motorist is the customer of everyone in the transportation industry. Like everyone else, bridge inspectors need to treat customers well by inconveniencing them as little as possible and protecting their safety. This means providing well thought out, clear, and effective traffic control measures.

Also consider pedestrians. If a walkway is to be closed, be sure it is properly signed and barricaded. Indicate an alternate route for the pedestrian, if necessary through or preferably around the work zone.

Training Each person whose actions affect inspection, maintenance and construction zone safety (from the upper-level management personnel to construction and maintenance field personnel) need training appropriate to the job decisions each individual is required to make. Only those individuals who are qualified by means of adequate training in safe traffic control practices and have a basic understanding of the principles established by applicable guidelines and regulations supervise the selection, placement, and maintenance of temporary traffic control devices in bridge safety inspection, maintenance, and construction areas.

Responsibility Legally and morally, it is the inspector's responsibility to follow the regulations and guidelines of the agency having jurisdiction.

The primary goal of good traffic control is safety – safety of the workers, motorists, and pedestrians. If there is an accident, the secondary goal is to be able to defend yourself and your employer. Accidents bring lawsuits. Lawsuits bring inquiries about who is responsible. Temporary traffic control is one thing that is investigated. Anything not done in accordance with published standards, regulations, and directives could bring blame upon whoever violated them. Being blamed for an accident is expensive and damaging.

Table of Contents

Chapter 2 Safety Fundamentals for Bridge Inspectors

2.4	Inspection Equipment		
	2.4.1	Equipment Necessity 2.4.1	
	2.4.2	Standard Tools2.4.1Tools for Cleaning2.4.3Tools for Inspection2.4.3Tools for Visual Aid2.4.4Tools for Measuring2.4.4Tools for Documentation2.4.4Tools for Access2.4.5Miscellaneous Equipment2.4.5	
	2.4.3	Special Equipment2.4.5Survey Equipment2.4.5Non-destructive Evaluation Equipment2.4.5Underwater Inspection Equipment2.4.5Other Special Equipment2.4.6	
	2.4.4	Recent Developments in Equipment2.4.6Rotary Percussion2.4.6Scour Measurement2.4.7Scour Monitoring2.4.7Side Scan Sonar2.4.7Multi-beam Sonar2.4.7Scanning Sonar2.4.7Veb-based Sour Monitoring2.4.7Portable depth Sounders with Transducers2.4.7Scour Monitoring Collar2.4.7Remote Camera2.4.10High Speed Underclearance Measurement System2.4.11Robots2.4.11Data Recording2.4.12Hardware2.4.12Software2.4.12	
	2.4.5	Primary Safety Concerns 2.4.13	

This page intentionally left blank.

Topic 2.4 Inspection Equipment

2.4.1				
Equipment Necessity	Several factors play a role in what type of equipment is needed for an inspection. Bridge location and type are two of the main factors in determining equipment needs. If the bridge is located over water, certain pieces of equipment such as life jackets and boats are necessary to have. Also, if the bridge is made of timber, then specific pieces of equipment like timber boring tools and ice picks are needed, whereas they are not necessary on a steel or concrete bridge. Another factor influencing equipment needs is the type of inspection. It is therefore important to review every facet about the bridge before beginning an inspection. A few minutes spent reviewing the bridge files and making a list of the necessary equipment can save hours of wasted inspection time in the field if the inspectors do not have the required equipment.			
2.4.2	—			
Standard Tools	In order for the inspector to perform an accurate and comprehensive inspection the proper tools are to be used. Standard tools that an inspector uses at the bridge site can be grouped into seven basic categories:			
	\blacktriangleright Tools for cleaning (see Figure 2.4.1)			
	 Tools for inspection (see Figure 2.4.2) 			
	> Tools for visual aid (see Figure 2.4.3)			
	Tools for measuring (see Figure 2.4.4)			
	Tools for documentation			
	Tools for access			
	Miscellaneous equipment			

Figure 2.4.1 Tools for Cleaning

CHAPTER 2: Fundamentals of Bridge Inspection TOPIC 2.4: Inspection Equipment



Figure 2.4.2 Tools for Inspection



Figure 2.4.3 Tools for Visual Aid



Figure 2.4.4 Tools for Measuring

Tools for Cleaning Tools for cleaning include:

- Wisk broom used for removing loose dirt and debris
- Wire brush used for removing loose paint and corrosion from steel members
- Scrapers used for removing corrosion or growth from member surfaces
- Flat bladed screwdriver used for general cleaning and probing
- Shovel used for removing dirt and debris from bearing areas

Tools for Inspection Tools for inspection include:

- Pocket knife used for general duty
- > Ice pick used for surface examination of timber members
- Hand brace and bits used for boring suspect areas of timber members
- > Timber boring tools used for internal examination of timber members
- Chipping hammer with leather holder (16 ounce geologist's pick) used for loosening dirt and rust scale, sounding concrete, and checking for sheared or loose fasteners
- Plumb bob used to measure vertical alignment of a superstructure or substructure member
- Tool belt with tool pouch used for convenient holding and access of small tools
- Chain drag used to identify areas of delamination on concrete decks
- Range pole / probe used for probing for scour holes

Tools for Visual Aid	Tools for visual aid include:		
		Binoculars - used to preview areas prior to inspection activity and for examination at distances	
	\triangleright	Flashlight - used for illuminating dark areas	
		Lighted magnifying glass (e.g., five power and 10 power) - used for close examination of cracks and areas prone to cracking	
		Inspection mirrors - used for inspection of inaccessible areas (e.g., underside of deck joints)	
	\triangleright	Dye penetrant - used for identifying cracks and their lengths	
Tools for Measuring	Tools f	or measuring include:	
		Pocket tape (six foot rule) - used to measure deficiencies and member and joint dimensions	
	\succ	25 foot and 100 foot tape - used for measuring component dimensions	
	۶	Calipers - used for measuring the thickness of a member beyond an exposed edge	
	\triangleright	Optical crack gauge - used for precise measurements of crack widths	
	\triangleright	Paint film gauge - used for checking paint thickness	
		Tiltmeter and protractor - used for determining tilting substructures and for measuring the angle of bearing tilt	
		Thermometer - used for measuring ambient air temperature and superstructure temperature	
		Four foot carpenter's level - used for measuring deck cross-slopes, approach pavement settlement and substructure alignment	
	\triangleright	D-Meter (ultrasonic thickness gauge) - used for accurate measurements of steel thickness	
		Electronic Distance Meter (EDM) - used for accurate measurements of span lengths and clearances when access is a problem	
	\triangleright	Line level and string line	
Tools for Documentation	Tools f	for documentation include:	
		Inspection forms, clipboard, and pencil - used for record keeping for most bridges	
	\succ	Note books - used for additional record keeping for complex structures	
	\succ	Straight edge - used for drawing readable sketches	
		Digital camera - used to provide digital images of deficiencies which can be downloaded and e-mailed for instant assessment	
		Chalk, kiel, paint sticks, or markers - used for member and defect identification for improved organization and photo documentation	
	۶	Center punch - used for applying reference marks to steel members for movement documentation (e.g., bearing tilt and joint openings)	
	>	"P-K" nails - Parker Kalon masonry survey nails used for establishing a reference point necessary for movement documentation of substructures and large cracks	

Tools for Access Some common tools for access include:

- > Ladders used for substructures and various areas of the superstructure
- Boat used for soundings and inspection; safety for over water work
- Rope used to aid in climbing
- ➤ Waders used for shallow streams

Tools for access are described in further detail in Topic 2.5.2.

Miscellaneous Equipment Miscellaneous equipment includes:

- "C"-clamps used to provide a "third hand" when taking difficult measurements
- Penetrating oil aids removal of fasteners, lock nuts, and pin caps when necessary
- > Insect repellent reduces attack by mosquitoes, ticks, and chiggers
- Wasp and hornet killer used to eliminate nests to permit inspection
- First-aid kit used for small cuts, snake bites, and bee stings
- Dust masks or respirators used to protect against inhalation in dusty condition or work around pigeon droppings
- Coveralls used to protect clothing and skin against sharp edges while inspecting
- Life jacket used for safety over water
- Cell phone used to call in emergencies
- > Toilet paper used for other "emergencies" (better safe than sorry)

2	Δ	3
	· •	

Special Equipment	For the routine inspection of a common bridge, special equipment is usually not necessary. However, with some structures, special inspection activities require special tools. These special activities are often subcontracted by the agency responsible for the bridge. These inspectors are familiar with the special equipment and its application.
Survey Equipment	Special circumstances may require the use of a transit, a level, an incremental rod, or other survey equipment. This equipment can be used to establish a component's exact location relative to other components, as well as an established reference point.
Non-destructive Evaluation Equipment	Non-destructive evaluation (NDE) is the in-place examination of a material for structural integrity without damaging the material. NDE equipment allows the inspector to "see" inside a bridge member and assess deficiencies that may not be visible with the naked eye. Generally, a trained technician is necessary to conduct NDE and interpret their results. For a more detailed description of NDE, refer to Topics 15.1.2, 15.2.2, and 15.3.2.
Underwater Inspection Equipment	Underwater inspection is the examination of substructure units and the channel below the water line. When the waterway is shallow, underwater inspection can be performed above water with a simple probe. Probing can be performed using a range pole, piece of reinforcing steel, a survey rod, a folding rule, or even a tree limb.

When the waterway is deep, an underwater inspection is performed by trained divers. This requires special diving equipment that includes a working platform, fathometer, air supply systems, radio communication, and sounding equipment. Refer to Topic 13.3 for a more detailed description of underwater inspection equipment.

Other Special Equipment An inspection may require special equipment to prepare the bridge prior to the inspection. Such special equipment includes:

- Air-water jet equipment used to clean surfaces of dirt and debris
- Sand or shot blasting equipment used to clean steel surfaces to bare metal
- > Burning, drilling, and grinding equipment

2.4.4

Recent Developments in Equipment

In addition to the standard and special equipment listed previously, there are new equipment and technology available to aid in bridge inspection. The developments in various types of advanced testing methods are described in Topics 15.1, 15.2 and 15.3. The following information represents some of the advances in inspection tools and data collection.

Rotary Percussion Rotary percussion is a method whereby a uniform tapping is produced by rolling a gear-toothed wheel on a concrete member to detect the presence of concrete deficiencies. This allows for the inspection of overhead and vertical surfaces to be done quickly, and is similar to using a chain drag for the inspection of horizontal surfaces. Advantages of rotary percussion testing tools include the ability to detect near-surface delaminations, quickness of testing, low equipment cost, relatively low level of user's skill required, and low sensitivity to the surroundings.



Figure 2.4.5 Rotary Percussion

Scour Measurement There is a specialized device used to measure the depth of scour during flood flows. It consists of a depth finder mounted on a water ski. The use of a water ski allows for depth readings to be taken in extremely fast flowing water and also allows for excellent maneuverability of the depth finder into locations under a bridge.

Scour Monitoring Side Scan Sonar

Side scan sonar is a specialized application of basic sonar theory. Although common for oceanographic and hydrographic survey work, side scan sonar has not been widely utilized for portable scour monitoring. Side scan sonar transmits a specially shaped acoustic beam to either side of the support craft, which allows for one of the most accurate systems for imaging large areas of channel bottom. A disadvantage to this method is that most side scan systems do not provide depth information.

Multi-beam Sonar

Multi-beam systems provide similar fan-shaped coverage to side scan systems, but output depths instead of images. Multi-beam sonar is typically attached to the surface vessel rather than being towed.

Scanning Sonar

Scanning sonar operates by rotating the transducer assembly, emitting a beam while the assembly (or "head") moves in an arc. Scanning sonar is performed by moving the transducer assembly, which allows it to be used from a fixed, stationary position.

Web-based Scour Monitoring

Scour monitoring software allows transportation engineers to predict, identify, prepare for, and record potentially destructive flooding events through a secure internet connection. This type of system identifies the occurrence of a flood event and collects and processes relevant bridge information, several sources of real-time hydrological data and any bridge scour monitoring device data. Transportation officials are able to efficiently dispatch emergency personnel, bridge safety inspectors, and maintenance workers before, during, and after a flood event affects a state's bridge inventory.

Portable Depth Sounders with Transducers

Portable depth sounders with transducers have been used to monitor real time scour at substructure units during major flood events. The deck elevations and scour depths of concern are indicated on the Scour Action Plan. If the scour reaches the critical depth specified, the bridge is closed.

Scour Monitoring Collar

The Magnetic Sliding Collar (MSC) is a scour monitoring device. The magnetic sliding collar device consists of a stainless steel pipe driven into the channel

bottom with a sliding collar that drops down the pipe as the scour progresses. The location of the collar is detected by the magnetic field created by magnets on the collar. Installations conducted in cooperation with state highway agencies demonstrated that this simple, low-cost instrument is adaptable to various field situations, and can be installed with the equipment and technical skills normally available at the district level of a state highway agency.



Figure 2.4.6 Scour Monitoring Collar

31



Figure 2.4.7 Scour Monitoring Collar Schematic

Remote Camera The basic components of a computer-based image system include: an imaging sensor, most commonly a solid-state camera; the image acquisition boards, which convert optical images into an array of digital information, representing the brightness values of the surface; and dedicated processor. The computer-based imaging system can provide two main types of information: spatial measurements and surface analysis. Spatial information encompasses two-dimensional or three-dimensional analysis, measurements and recognition. The surface analysis provides information regarding the color or gray-scale attributes of the target. For example, imaging systems are able to distinguish a flaw from the rest of the surface, and determine size, shape, location and even smallest color attributes of the deficiency. The field of view can be processed in a fraction of a second and can be on the order of 200 to 500 times the size of the smallest feature of interest.

This system works well in a clean environment, however if the item/member is very dirty or has debris surrounding it, cleaning may need to be performed prior to using a remote camera.



Figure 2.4.8 Remote Camera

High Speed Underclearance Measurement System

The high speed underclearance measurement system can mount on any vehicle with a trailer hitch receiver. The system measures the underclearance of a bridge at normal highway speeds. Along with the underclearance data, the GPS location is gathered. Software is used for the data acquisition, display and analysis. The bridge beam height is read to the nearest tenth of a foot. The GPS information can be pasted into a map program to obtain the structure location for future reference.



Figure 2.4.9 High Speed Underclearance Measurement System

RobotsRobotic devices for many applications are being developed by university
researchers. High level and underwater bridge inspection are among these
applicatons.One example is the serpentine robot being developed that possesses multiple joints
that give it a superior ability to flex, reach, and approach every point on the bridge.
This robot is under development.**Laser Scanning**Laser scanning technology can create accurate and complete 3D as-built models
quickly and safely. These digital models are automatically combined with CAD
design models to allow generation of "as-built" drawings for existing structures.
This method can replace tedious field measurements for rehabilitation projects.

Data Recording The majority of bridge inspectors use pencil and paper to record deficiencies on a bridge. They usually take a copy of the last inspection notes or report and "mark-up" changes since the last inspection. The inspectors input the current findings into the bridge owner's software and the inspection is updated.

Many State agencies are using Electronic Data Collection for bridge inspection.

Hardware

Data recording hardware can include regular office computers, notebook computers or tablet PCs (see Figure 2.4.10) Some versions of these devices have been made to be more rugged and even "wearable" for use in the field.

Software

Specialized software packages can provide a comprehensive set of solutions to manage, inspect, maintain and repair bridges. They allow the user to maintain a comprehensive asset inventory database, collect inspection data from electronic devices, keep history of inspection and maintenance records, assign inspection and maintenance requirements to each structural component, automatically generate inspection reports, and offer decision support.

AASHTO Pontis suports databases on bridge inspection and management. Many bridge owners use AASHTO Pontis based software and have developed programs to address their specific needs.



Figure 2.4.10 Tablet PC Used to Collect Inspection Data

2.4.5 Primary Safety Concerns Proper inspection equipment plays a key role in maintaining the safety of the traveling public and the inspectors. Inspectors who do not have the right equipment, may attempt to use an alternate piece of equipment that is not really designed for the job. Using whatever equipment is at hand, in an attempt to save time and money, can prove dangerous for the inspection team as well as the public. The best way to avoid these circumstances is to ensure the inspectors have the proper equipment for the job and that the equipment is serviced or replaced periodically. This responsibility lies not only with the inspector or team leader but also their employer. It is important that the employer make every effort to properly equip their inspection teams. Also, the inspector needs to be familiar with every piece of equipment and how to use and operate it properly and safely.

Safety fundamentals for bridge inspectors is presented in detail in Topic 2.2.

This page intentionally left blank
Table of Contents

Chapter 2 Safety Fundamentals for Bridge Inspectors

2.5	Methods of Access			
	2.5.1	Introduction		
	2.5.2	Types of Access Equipment		
		Ladders		
		Rigging		
		Scaffolds		
		Boats or Barges		
		Climbers		
		Floats		
		Bosun (or Boatswain) Chairs/Rappelling		
		Free Climbing		
		Permanent Inspection Structures		
		Catwalks		
		Traveler		
		Handrails		
		Inspection Robots		
	2.5.3	Types of Access Vehicles		
		Manlift		
		Scissors Lift		
		Bucket Truck		
		Under Bridge Inspection Vehicle		
	2.5.4	Method of Access and Cost Efficiency		
	2.5.5	Safety Considerations		
		Access Equipment		
		Access Vehicles		

This page intentionally left blank.

Topic 2.5 Methods of Access

2.5.1	
Introduction	The two primary methods of gaining access to hard to reach areas of a bridge are access equipment and access vehicles. Common access equipment includes ladders, rigging, and scaffolds, while common access vehicles include manlifts, bucket trucks, and under bridge inspection vehicles. In most cases, using a manlift or bucket truck will be less time consuming than using a ladder or rigging to inspect a structure. The time saved, however, is normally offset by the higher costs associated with operating access vehicles.
2.5.2	
Types of Access Equipment	The purpose of access equipment is to position the inspector close enough to the bridge component so that a "hands-on" inspection can be performed. The following are some of the most common forms of access equipment used in bridge inspection.
Ladders	Ladders are used for inspecting the underside of a bridge or for inspecting substructure units. However, a ladder is used only for those portions of the bridge that can be reached safely, without undue leaning or reaching. The proper length of the ladder is determined by using it at a four vertical to one horizontal angle. When set up at the proper angle (1 horizontal to 4 vertical), the inspector is able to reach out horizontally, grasp the rung while keeping his or her feet at the base of the ladder (see Figure 2.5.1).



Figure 2.5.1 Inspectors using a Ladder with the Proper 1H to 4V Ratio

Ladders are also used to climb down to access members of the bridge. The hookladder, as it is commonly referred to, is fastened securely to the bridge framing (see Figure 2.5.2).



Figure 2.5.2 Inspector using a Hook-ladder

When using a hook-ladder, the inspector is tied off to a separate safety line, independent of the ladder.

Rigging of a structure consists of cables and platforms. Rigging is used to gain access to floor systems and main load-carrying members in areas where access by other means is not feasible or where special inspection procedures are required (e.g., NDE of pins). Rigging is often used when ladders or other access equipment cannot reach a given location (see Figures 2.5.3 and 2.5.4). Rigging is a good choice for a load-posted bridge that does not have the capacity to support an inspection vehicle.

> Rigging does not interfere with traffic on the bridge and can be used in high traffic situations where lane closures are intolerable, and on toll facilities to avoid loss of revenue. Rigging may not be an option if there is not enough clearance to avoid interfering with passing features below the bridge.

Rigging

CHAPTER 2: Fundamentals of Bridge Inspection TOPIC 2.5: Methods of Access



Figure 2.5.3 Rigging for Substructure Inspection



Figure 2.5.4 Rigging for Superstructure Inspection

Scaffolds

Scaffolds provide an efficient access alternative for structures that are less than 40 feet high and over level ground with little or no traffic nearby (see Figure 2.5.5). The Occupational Safety and Health Administration (OSHA) has specific requirements when working on scaffolding. Scaffolds may take longer to set up than it takes to inspect the bridge. For this reason, scaffolding is not normally used for bridge inspections.





Boats or Barges A boat or barge may be needed to gain access to structures over water. A boat can be used for inspection, as well as providing access to areas for taking photographs. Also, a safety boat is required when performing an inspection over water (see Figure 2.5.6).



Figure 2.5.6 Inspection Operations from a Barge

A barge may also be used in combination with other access equipment or vehicles to perform an inspection. The barge may be temporarily anchored in place to provide a platform for a manlift or mobilization for underwater inspections.

Climbers are mobile inspection platforms or cages that "climb" steel cables or truss members (see Figure 2.5.7). They are well suited for the inspection of high piers and other long vertical faces of bridge members.



Figure 2.5.7 Climber

Climbers

Floats

A float is a wood plank work platform hung by ropes (see Figure 2.5.8). Floats are generally used for access in situations where the inspector will be at a particular location for a relatively long period of time.



Figure 2.5.8 Inspector Using Float

Bosun (or Boatswain) Chairs / Rappelling

Bosun (or boatswain) chairs are suspended with a rope and can carry one inspector at a time. They can be raised and lowered with block and tackle devices. Rappelling is a similar access method to the Bosun chair but utilizes different equipment and techniques (see Figure 2.5.9). However, both methods require the use of independent safety lines.



Figure 2.5.9 Inspector Rappelling Substructure Unit

Free Climbing On structures, where other methods of access are not practical, inspectors climb on the bridge members to gain access (see Figure 2.5.10). Safety awareness is of the utmost importance when utilizing this technique. When using this method, the inspector is tied off to the bridge using an independent safety harness and lanyard.



Figure 2.5.10 Climbing

Permanent Inspection Structures

On some structures, inspection access is included in the design and construction of the bridge. These are typically found on long span structures or more complex designs. Although these inspection platforms only give access to a limited portion of the bridge, they do provide a safe and effective means for the inspector to work. The following are some examples of permanent inspection structures.

Catwalks

A catwalk is an inspection platform typically running parallel to the girders under the superstructure (see Figure 2.5.11). Catwalks can be used to inspect parts of the deck, superstructure and some portions of the substructure. The range of inspection area is limited to those locations near the catwalk.



Figure 2.5.11 Catwalk

Traveler

A traveler is another permanent inspection platform similar to a catwalk except that it is movable. A traveler platform is typically perpendicular to the girders and the platform runs on a rail system between substructure elements (see Figure 2.5.12). Having the platform perpendicular to the girders allows the inspectors a wider range of movement and enables them to see more of the superstructure elements.



Figure 2.5.12 Traveler Platform

Handrails

Handrails are also used to aid an inspector. Handrails can be used in a number of different locations on the bridge. On the main suspension cables, on top of the pier caps, and on the girder web are just a few locations where handrails may be built (see Figures 2.5.13 and 2.5.14). Handrails are typically provided to assist the inspector when free climbing on the bridge and give the inspector a place to secure their lanyard and safety harness.



Figure 2.5.13 Handrail on Girder Bridge



Figure 2.5.14 Handrail on Suspension Bridge

Inspection Robots Currently, efforts are being made for robots to be used for inspection purposes. Though still early in the development stage, robots may prove to be an important addition to the inspector's access equipment. Although a robot can never replace a qualified inspector, it can provide information that may not be visible to the human eye. A robot equipped with sonar capabilities can detect internal flaws in bridge members. Also, a robot can be used in situations that are too difficult to reach or extremely dangerous for a human.

2.5.3

Types of Access Vehicles There are many types of vehicles available to assist the inspector in gaining access for "hands-on" inspection of bridge members. The following are some of the most common types of access vehicles used in bridge inspection.

Manlift A manlift is a vehicle with a platform or bucket capable of holding one or more inspectors. The platform is attached to a hydraulic boom that is mounted on a carriage. An inspector "drives" the carriage using controls in the platform. This type of vehicle is usually not licensed for use on highways. However, some manlifts are nimble and can operate on a variety of terrains. Although four wheel drive models are available, manlifts are limited to use on fairly level terrain. Manlifts come in a number of different sizes with vertical reaches ranging from 40 feet to over 170 feet (see Figure 2.5.15).



Figure 2.5.15 Manlift

Scissors Lift Scissor lifts may be used for bridge inspections with low clearance between the bridge and underpassing roadway. Scissor lifts have a typical maximum vertical reach of 20 feet. These lifts are designed for use on relatively level ground (see Figure 2.5.16).



Figure 2.5.16 Scissor Lift

Bucket Truck A bucket truck is similar to a manlift. However, a bucket truck can be driven on a highway, and the inspector controls bucket movement (see Figure 2.5.17). As with the manlift, a bucket truck needs to be used on fairly level terrain. Bucket trucks have a number of different features and variations:

- Lift capability varies 25 to 50 feet.
- Rotating turret turning range (i.e., the rotational capability of the turret) varies with each vehicle.
- Telescoping boom some booms may be capable of extending and retracting, providing a greater flexibility to reach an area from a given truck location.
- Multiple booms some bucket trucks have more than one boom, and provide reach up to 50 feet.
- Outriggers bucket trucks that offer extended reach and turning range have outriggers or supports that are lowered from the chassis of the vehicle to help maintain stability.
- Truck movement some vehicles offer stable operations without outriggers and can move along the bridge during inspection activities. Vehicles that require outriggers for stable operations cannot be moved during the inspection unless the outriggers have wheels.



Figure 2.5.17 Bucket Truck

A track-mounted man-lift provides access to areas with rough terrain that a conventional bucket truck would not be able to navigate (see Figures 2.5.18 and 2.5.19). By utilizing rubber tracks, track-mounted man-lifts can be operated in water, climb 35 degree slopes, traverse 25 degree side slopes, and navigate wet and muddy terrain.



Figure 2.5.18 Track-mounted Man-lift in a Stream



Figure 2.5.19 Track-mounted Man-lift on a Slope

Under Bridge Inspection Vehicle

An under bridge inspection vehicle is a specialized bucket truck with an articulated boom designed to reach under the superstructure while parked on the bridge deck. Usually the third boom has the capacity for extending and retracting, allowing for greater reach under a structure. Some of the larger under bridge inspection vehicles have four booms, allowing an even greater reach (see Figure 2.5.20).



Figure 2.5.20 Under Bridge Inspection Vehicle with Bucket

Variations and options available on different models include:

- Capacity Some under bridge inspection vehicles have a two or three person bucket on the end of the third boom. Other models are equipped with a multiple-person platform on the third boom with a ladder on the second boom. Still other models may have the capability of interchanging a bucket and a platform in the shop.
- Platform The platform is lowered by an articulated boom and can then telescope out to provide inspection access to a wide range of the superstructure and substructure. The inspector is now free to walk from beam to beam without having to reposition the platform (see Figure 2.5.21). This combination allows for an efficient and thorough inspection.
- Telescoping second boom Some under bridge inspection vehicle models have a second boom that can extend and contract, providing greater movement in the vertical direction.
- Articulated third (or fourth) boom Some under bridge inspection vehicle models have a small third or fourth boom that allows for greater vertical movement under the structure. This option is particularly useful on bridges with deep superstructure members.



Figure 2.5.21 Under Bridge Inspection Vehicle with Platform

2.5.4			
Method of Access and Cost Efficiency	In most cases, even the most sluggish lift device will be quicker than using a ladder, rigging or free climbing to inspect a structure. The time saved, however, needs to offset the higher costs associated with obtaining and operating an access vehicle.		
	In assessing the time-saving effectiveness of an access vehicle, the following questions need to be answered:		
	Can the bridge be safely inspected by other reasonable methods?		
	What types of access vehicle or access equipment are available?		
	How much of the bridge can be inspected using the access vehicle?		
	How much of the bridge can be inspected from one setup of the access vehicle?		
	> How much time does it take to inspect at each setup?		
	How much time does it take to move from one setup to the next?		
	> Does the vehicle require an independent operator or driver other than the inspector?		
	> Will the use of the access vehicle require special traffic control?		
	> Can the bridge carry the weight of an inspection vehicle?		
	> What are the associated costs of using a bridge inspection access vehicle?		
	The inspection time, safety and vehicle costs can then be compared to standard access equipment.		
2.5.5			
Safety Considerations	Safety is the primary concern on any job site, not only of the workers but of the public as well. The equipment and vehicles being used also have safety considerations.		
Access Equipment	Before the bridge inspection begins, an equipment inspection is performed. As a minimum, inspect access equipment as per the manufacture's guidelines. Using faulty equipment can lead to serious accidents and even death. Check the equipment and verify that it is in good working condition with no defects or problems. If rigging or scaffolding is being used, check to ensure that it was installed properly and that the cables and planks are secured tightly. Use OSHA-approved safety harnesses with shock absorbing lanyards when using access equipment.		
Access Vehicles	If the inspector is not familiar with the inspection vehicle being used, then take the time required to become accustomed to its operation. In some cases, formal operator training may be necessary or required. When operating any inspection vehicle, always be aware of any overhead power lines or other hazards that may exist. It is also important to be aware of any restrictions on the vehicle, such as weight limits for the bucket, support surface slope limits, and reach restrictions. Always be alert to your location. Do not extend the boom out into unsafe areas such as unprotected traffic lanes or near electrical lines. Use OSHA-approved safety harnesses with shock absorbing lanyards when using access vehicles.		

Table of Contents

Chapter 3 Basic Bridge Terminology

3.1	Basic Bridge Terminology			
	3.1.1	Introduction		
	3.1.2	NBIS Structure Length		
	3.1.3	NBIS Bridge Length 3.1.2		
	3.1.4	Major Bridge Components		
	3.1.5	Basic Member Shapes3.1.3Timber Shapes3.1.3Planks3.1.4Beams3.1.4Piles/Columns3.1.5Concrete Shapes3.1.5Cast-in-Place Flexural Shapes3.1.6Precast Flexural Shapes3.1.7Axially-Loaded Compression Members3.1.9Iron Shapes3.1.10Cast Iron3.1.10Steel Shapes3.1.11Built-up Shapes3.1.11Built-up Shapes3.1.17		
	3.1.6	Connections.3.1.18Pin Connections.3.1.18Riveted Connections3.1.20Bolted Connections3.1.21Welded Connections.3.1.22Pin and Hanger Assemblies.3.1.22Splice Connections.3.1.23		
	3.1.7	Decks3.1.24Deck Purpose3.1.24Deck Types3.1.25Deck Materials3.1.26Timber Decks3.1.27Concrete Decks3.1.27Steel Decks3.1.28Fiber Reinforced Polymer (FRP) Decks3.1.29Wearing Surfaces3.1.30		

	Deck Appurtenances, Signing and Lighting
	Deck Joints
	Drainage Systems
	Traffic Safety Features
	Sidewalks and Curbs
	Signing
	Lighting
3.1.8	Superstructures
	Superstructure Purpose
	Superstructure Types
	Slab Bridges 3.1.37
	Single Web Beam/Girder Bridges 3.1.37
	Trusses
	Arches
	Rigid Frames 3.1.43
	Cable-Supported Bridges
	Movable Bridges
	Floating Bridges
	Superstructure Materials
	Primary Members
	Secondary Members 3.1.48
3.1.9	Bearings
	Bearing Purpose
	Bearing Types
	Bearing Materials
	Bearing Elements
2 1 10	0.1.4.4
3.1.10	Substructure
	Substructure Purposes
	Substructure Types
	Abutments
	Piers and Bents
	Substructure Materials
	Substructure Elements
3 1 1 1	Culverts 3161
	Culvert Purpose
	Culvert Materials
	Culvert Types 3 1 61
	Rigid Culverts
	Flexible Culverts 3 1 67